# INTERSECTION CONTROL EVALUATION REPORT <br> Interstate 5 at Cook Road - Milepost 232.83 

September 2023

1. This Intersection Control Evaluation (ICE) Report was prepared by Design Consultant staff working under my direct supervision, consistent with the requirements of WSDOT Design Manual Chapters 300 and 1300.

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## Introduction

This report summarizes the Intersection Control Evaluation (ICE) completed for the Interstate 5 (I-5) Interchange at Cook Road in Skagit County, Washington. The interchange location and vicinity map is shown in Figure 1. The evaluation was conducted based on the guidelines set in Chapter 1300.05(3) Intersection Control Evaluation Section of the Washington State Department of Transportation (WSDOT) Design Manual M 22-01.21 (September 2022).


Figure 1. Intersection Vicinity Map
As shown in Figure 1, the ICE reviews the Cook Road intersections of the I-5 southbound ramps, I-5 northbound ramps, and Old Highway 99 with consideration for the existing railroad crossing located approximately 75 feet east of the Cook Road/Old Highway 99 N intersection operated by Burlington Northern Santa Fe Corporation (BNSF). This analysis evaluates 4 alternatives for the intersections in addition to the No Action Alternative which are summarized below.

Table 1. Description of Alternatives Evaluated

| Intersection | No Action | Alternative 1: Roundabout | Alternative 2: <br> Traffic Signal | Alternative 3: <br> Traffic Signal with added NBR at Old Highway 99 N | Alternative 4: <br> Traffic Signal \& single controller |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1. I-5 southbound ramps/Cook Rd | Two-Way Stop Controlled | Single Lane Roundabout | Traffic Signal (existing channelization) | Traffic Signal (existing channelization) | Traffic Signal (existing channelization) |
| 2. I-5 northbound ramps/Cook Rd | Two-Way Stop Controlled | Single Lane Roundabout | Traffic Signal with added NBR | Traffic Signal with added NBR | Traffic Signal with added NBR, single controller with Old Highway 99 N |
| 3. Old Highway 99 N /Cook Rd | Traffic Signal | Multilane Roundabout | Added EBT lane | Added EBT and NBR lanes | Added EBT and NBR lanes, single controller with Old Highway 99 N |

## Recommendation:

The recommendation for the intersection improvements is the Alternative 3 traffic signal alternative. This alternative would include the following:

- Install traffic signals at the 2 ramp intersections,
- Add a northbound right turn lane at the I-5 northbound ramp intersection,
- Widen Cook Road to include an additional eastbound through lane east of the I-5 northbound ramp intersection to east of Green Road, and
- Add a northbound right-turn lane at the Old Highway 99 N intersection.

The following documents the 5 -step ICE screening process that was coordinated with WSDOT to evaluate the alternatives and determine the best possible intersection type and design. The steps include:

1. Background and Project Needs
2. Feasibility
3. Operational and Safety Performance Analysis
4. Alternatives Evaluation
5. Selection

## Step 1: Background and Project Needs

The following section summarizes the existing conditions of the Cook Road corridor in the vicinity of the $\mathrm{l}-5$ ramps and Old Highway 99 N intersections and the adjacent at-grade railroad crossing as well as the project needs, methodology used for analysis, and comparison of the alternatives.

## Existing Conditions

The project area includes the intersections of Cook Road with the I-5 southbound ramps, I-5 northbound ramps, and Old Highway 99 with consideration for the existing BNSF railroad crossing located approximately 75 feet east of the Cook Road/Old Highway 99 N intersection. The primary roadways in the study area are described below.

- Cook Road is predominantly a two-lane roadway classified as a Major Collector by Skagit County with an estimated annual average daily traffic (AADT) of approximately 14,500 vehicles in the study area ${ }^{1}$.
- Old Highway $99 \mathbf{N}$ is a two-lane roadway with a general speed limit of 50 miles per hour (mph), providing an alternative route to $\mathrm{I}-5$. In the vicinity of the project area, the speed limit is reduced to 35 mph .
- The l-5 Ramps - The annual average daily traffic (AADT) in the study area was reviewed based on WSDOT's Traffic Count Database for the I-5 ramps showing approximately 3,000 ADT on each of the ramps to/from I-5 north of Cook Road (i.e. the southbound off-ramp and northbound on-ramp). South of Cook Road, the AADT of the l-5 ramps are approximately 5,000 and 6,000 ADT for the southbound on-ramp and northbound off-ramp, respectively. The volumes show the primary travel patterns in the vicinity of the Cook Road study intersections is to/from the south.

Each intersection and the BNSF railroad crossing are described below. The existing weekday AM and PM peak hour traffic volumes as well as the traffic control and channelization are included on Figure 2.

1. I-5 Southbound Ramps/Cook Road - This intersection is an existing two-way stopcontrolled intersection with the southbound approach being stop controlled and free movements east-west along Cook Road. All approaches are a single shared lane with Cook Road being a two-lane road and the l-5 Ramp being one-lane, one-way southbound. The peak hour total entering volumes (TEV) at this intersection are approximately 900 vehicles in both the AM and PM peak hours. No non-motorized facilities exist at this intersection.
2. l-5 Northbound Ramps/Cook Road - This intersection is an existing two-way stop-controlled intersection with the northbound approach being stop controlled and free movements eastwest along Cook Road. All approaches are a single shared movement lane with Cook Road being a two-lane road and the l-5 Ramp being one-lane, one-way northbound. The peak hour TEV at this intersection is approximately 1,350 and 1,430 vehicles in the AM and PM peak hours, respectively. No non-motorized facilities exist at this intersection.

[^0]3. Old Highway 99 N/Cook Road - This intersection is an existing traffic signal. The eastbound and westbound approaches along Cook Road as well as the northbound approach along Old Highway 99 N include a left-turn lane and shared through/right-turn lane. The southbound approach along Old Highway 99 N includes separate left, through, and right-turn lanes. The peak hour TEV at this intersection is approximately 1,740 and 2,035 vehicles in the AM and PM peak hours, respectively. Signalized pedestrian crossings are provided across all legs of the intersection.
4. At-Grade BNSF Crossing - The existing at-grade rail crossing is located approximately 75 feet east of the Old Highway 99 N/Cook Road intersection. The crossing consists of one eastbound and two westbound lane. The two westbound lanes consist of the left-turn and shared through/right-turn lanes of the westbound approach to the Old Highway 99 N/Cook Road intersection. No existing pedestrian facilities are present at the rail crossing. The crossing includes overhead warning lights, automatic gates, and a interconnect with the traffic signal at Old Highway 99 N/Cook Road to provide rail preempt at this intersection. The railroad crossing averaged 18 trains per day in the first half of 2023 (4 Amtrak and 14 nonAmtrak). Crossing blockages vary greatly but generally range between 2 to 7 minutes.

No transit facilities are provided within the study area.


Figure 2: Existing Weekday AM and PM Peak Hour Traffic Volumes and Traffic Control and Channelization at Study Intersections

## Project Needs

The project area currently experiences heavy peak hour congestion, largely stemming from the demand to/from I-5. In particular, demand during the PM peak hour often exceeds the capacity of the
system and results in queueing from the Old Highway 99 N/Cook Road intersection, through the I-5 northbound ramps intersection, and occasionally onto I-5 mainline. Gate closures at the railroad crossing increase this congestion. As described previously, the highest demand is to/from the south and east. The Comprehensive Plan indicates the traffic volumes at the study area intersections are forecast to continue to grow along with continued increase in impacts associated with the proximity of the railroad crossing as well as continued growth in train activity. In 2017, the County completed a corridor study for Cook Road that evaluated short- and long-term solutions for the project area. As a result of that study, the County secured a grant through the National Highway Freight Program (NHFP) to design and build improvements matching Alternative 2 in this ICE. Long term solutions for the area have been identified in the Comprehensive Plan to include a grade separated railroad crossing.

After securing the NHFP grant, and in coordination with WSDOT, it was determined that an ICE would be required prior to starting design of any improvements. This ICE reviews interim improvements that can improve operations prior to the installation of the long-term improvement. The interim improvements include 4 Action alternatives ( 1 roundabout and 3 traffic signal options).

## Intersection Traffic Control Alternatives

Four intersection control alternatives have been reviewed. Figure 3 provides a conceptual layout of the different alternatives. The detailed concepts are provided as well in Appendix A. The alternatives are described below in detail as well as previously summarized in Table 1.

## Alternative 1: Roundabout

The roundabout alternative assumes the intersections are reconstructed and converted from the current traffic control (either side-street stop-controlled or signal) to roundabouts. Each intersection is described below:

1. I-5 southbound ramps/Cook Rd: Single lane roundabout
2. I-5 northbound ramps/Cook Rd: Single lane roundabout
3. Old Highway 99 N /Cook Rd: Multilane roundabout with 2 circulating lanes and the following lane configurations:

- Eastbound left/through and through/right turn lanes
- Westbound, Northbound, and Southbound approaches include a separate left turn lane and shared through/right lane
- Single receiving lanes on all approaches with the exception of the east leg


## Alternative 2: Traffic Signal

The Alternative 2 traffic signal alternative assumes the two l-5 ramp intersections are converted from side-street stop-controlled to traffic signals. Each intersection is described below:

1. I-5 southbound ramps/Cook Rd: Traffic signal maintaining existing channelization.
2. l-5 northbound ramps/Cook Rd: Traffic signal with the addition of a northbound right turn lane on the northbound off-ramp resulting in a single shared left/through/right lane and separate right turn lane. Additionally, there is an added eastbound receiving lane east of the intersection along Cook Road.
3. Old Highway $99 \mathbf{N} /$ Cook Rd: Traffic signal with the addition of an eastbound through lane and eastbound receiving lane east of the intersection along Cook Road.

## Alternative 1: Roundabout



Alternative 2: Traffic Signal


Alternative 3 and 4*: Traffic Signal \& added NBR at Old Hwy 99 N


* Alternative 4 also includes the modification for a single controller for the intersections of Old Highway 99 N and I-5 Northbound Ramps along Cook Road.

Figure 3: Alternative Concepts

## Alternative 3: Traffic Signal \& added NBR at Old Hwy 99 N

The Alternative 3 traffic signal alternative is consistent with Alternative 2 which assumes the two l-5 ramp intersections are converted from side-street stop-controlled to traffic signals but includes additional channelization modifications. The additional modifications with Alternative 3 relative to Alternative 2 are described below.

1. l-5 southbound ramps/Cook Rd: Consistent with Alternative 2.
2. I-5 northbound ramps/Cook Rd: Consistent with Alternative 2.
3. Old Highway 99 N /Cook Rd: Alternative 2 modifications as well as the addition of a northbound right turn lane providing separated northbound left, through, and right turn lanes for the northbound approach. This alternative provides continued movement of northbound left and through vehicles during railroad crossing events.

## Alternative 4: Traffic Signal \& Single Controller

The Alternative 4 traffic signal alternative is consistent with Alternative 3 which assumes the two l-5 ramp intersections are converted from side-street stop-controlled to traffic signals and channelization modifications. No additional channelization changes are proposed with Alternative 4 relative to Alternative 3; however, the Alternative 4 traffic signals at the intersections of Old Highway 99 N and I 5 northbound ramps along Cook Road would operate on a single controller.

## Methodology

The following section provides an overview of the methodology used to analyze the intersection control alternatives.

## Horizon Years

The horizon years include an approximate opening year of 2028 and a 2045 design year. The design year is consistent with the County's travel demand model.

## Performance Measures

Level of service (LOS), volume to capacity ratio (V/C), and queuing are used as performance measures to compare each alternative. Each alternative is evaluated with and without the impact of the railroad crossing. In addition, a discussion of safety performance was conducted for each alternative. Traffic operation assumptions include:

- All Alternatives -
- For roundabout and traffic signal-controlled intersections, LOS is measured in average delay per vehicle and is reported for the intersection as a whole. At unsignalized side-street, stop-controlled intersections, LOS is measured by the average delay on the worst-movement of the intersection. Traffic operations for an intersection can be described alphabetically with a range of levels of service (LOS A through $F$ ), with LOS A indicating free-flowing traffic and LOS F indicating extreme congestion and long vehicle delays. Appendix B contains a detailed explanation of LOS criteria and definitions.
- Skagit County's Comprehensive Plan (2016) identifies a LOS standard of LOS D at intersections which includes the Old Highway 99 N/Cook Road intersection. WSDOT identifies a LOS C standard along l-5 in the vicinity of the Cook Road ramps.
- During the Design Year condition, the peak hour factor (PHF) for the design year conditions was set to 1.0 for both the No Action and Action Alternatives per the WSDOT guidelines ${ }^{2}$. The existing PHF was maintained for the Opening Year conditions.

[^1]- To evaluate a train event, a 5-minute gate-down event was assumed for all alternatives (No Action and Action). The train event was evaluated for the signalized and roundabout alternatives as described below.
- Signalized Alternatives and No Action - Evaluated based on guidelines found in WSDOT Synchro \& Simtraffic Protocol (August 2018). Note that operations were reviewed in Synchro 11 (a software program that uses HCM methodology to evaluate intersection LOS and average vehicle delay); however, in order to accurately reflect the differences in operations associated with a train event, the operations and queues were reported using SimTraffic.
- Roundabout Alternatives - Evaluated using Sidra 9.0 network analysis. Sidra model settings used for the roundabout evaluation were based on the guidelines in the WSDOT Sidra Policy Settings (October 2020). Additionally, for roundabouts, a v/c ratio of 1.0 or less for each lane group is recommended. The environmental factor (EF) was set to 1.1 during the Opening Year condition and 1.0 during the Design Year condition.


## Traffic Volume Forecasts

Future (2028) Opening Year weekday AM and PM peak hour traffic volumes were forecast by applying an annual growth rate to existing traffic volumes. An annual growth rate of 1.0 percent was applied to existing study intersection traffic volumes to estimate 2028 horizon year traffic growth based on review of historical growth in the area as well as review of growth anticipated between the County's 2018 and 2045 Travel Demand Models.

The future Design Year weekday AM and PM peak hour traffic volume forecasts are based on Skagit County's Travel Demand Model. The model contains estimates of future land use growth in the region. The travel demand models forecast weekday PM peak hour conditions. Weekday AM peak hour traffic volumes are developed through their relationship with the existing weekday PM peak hour traffic counts. Note that adjustments were made for reasonableness.

The Opening and Design Year weekday AM and PM peak hour traffic volumes are shown in Figure 4. The long-term travel demand model anticipates limited growth to/from I-5, but rather the growth is concentrated along the Cook Road and Old Highway 99 N corridors.


Figure 4: Future Weekday AM and PM Peak Hour Volumes

## Roundabout Geometrics

For the roundabout alternative (Alternative 1), key features include:

- Roundabout geometrics developed in accordance with WSDOT Design Manual, Section 1320 and NCHRP Report 1043
- Dual 15 -foot circulating lanes or 20-foot single circulating lanes.
- Central Island diameter is approximately 80 feet for multilane roundabouts or 75 feet for single lane roundabouts.


## Signal Timing Parameters

For the signalized alternatives (Alternatives 2-4), the following signal timing parameters were assumed for the analysis:

- Actuated-coordinated control (coordinated along the Cook Road corridor). The traffic signals under Alternatives 2 and 3 have a 120-second cycle length. The traffic signals under Alternative 4 (including the single controller for the intersections of l-5 northbound ramps and Old Highway 99 N along Cook Road) have a 140-second cycle length.
- Splits and offsets were optimized
- Left-turn movements are flashing yellow


## Design Vehicle

For both the traffic signal and roundabout alternatives, design concepts were developed to accommodate WB-67 truck turning movements.

## Step 2: Feasibility

The alternatives were analyzed for feasibility based on the factors included in Chapter 1300 of the WSDOT Design Manual as well as consideration of site-specific issues. Table 2 summarizes the feasibility comparison of each alternative. Based on the feasibility review, no alternatives were eliminated, and all Action Alternatives are reviewed further in Step 3 below relative to the No Action condition.

## Table 2. Alternative Feasibility Comparison

| Factor | Alternative 1: Roundabout | Alternative 2: <br> Traffic Signal | Alternative 3: <br> Traffic Signal with added NBR at Old Highway 99 N | Alternative 4: <br> Traffic Signal \& single controller |
| :---: | :---: | :---: | :---: | :---: |
| Right-of-Way Impacts | Right-of-way acquisition would be necessary at the Old Highway 99 intersection, including the southwest, southeast, and northeast corners. In addition, additional ROW may be required east of the railroad crossing to allow for roadside grading and provide drainage ditches. Due to the larger amounts of added/replaced impervious surface required for Alternative 1, flow control and water quality facilities would likely be more significant, requiring the potential need for additional right-of-way to locate the facilities. <br> Alternative 1 may also impact the north access point of the gas station located on the southwest corner of the Old Highway 99 intersection. It is anticipated the splitter islands of the northbound approach to the roundabout will extend through this access. Impacts could include restricting the access to right-in/right-out only. | Alternative 2 would require a similar Right-of-way acquisition as Alternative 1. It is anticipated that Alternative 2 will have slightly less added/impervious surface and may require slightly smaller stormwater facilities. | Alternative 3 is anticipated to have similar Right-of-way acquisition needs as Alternative 2. However, the added northbound right-turn lane at the Old Highway 99 intersection may require additional ROW as well as increase the amount of new/replaced impervious surfaces. This may increase the size of the needed water quality and flow control facilities and associated Right-of-way needs. | Right-of-way acquisition under Alternative 4 would be similar to that of Alternative 2. |
| Sensitive Area Impacts | No documented wetlands are present near the project limits. This alternative would have a higher amount of new/replaced impervious surface relative to other alternatives, requiring more substantial detention and treatment of stormwater. | No documented wetlands are present near the project limits. This alternative is anticipated to result in the lowest amount of new/replaced impervious surface that would require detention and treatment of stormwater. | No documented wetlands are present near the project limits. This alternative would result in a higher amount of new/replaced impervious surfaces than Alternative 2, requiring larger water quality and flow control facilities. | No documented wetlands are present near the project limits. This alternative would result in similar amounts of new/replaced impervious surfaces as Alternative 2. |
| Design Constraints | The major design constraints for Alternative 1 include the topography of the project area, the proximity of the l-5 bridge to the SB and NB ramp terminal intersections, and the proximity of the railroad crossing to the Old Highway 99 intersection. Grades between the I-5 NB ramp terminal intersection and the Old Highway 99 intersection are anticipated to increase from approximately $4 \%$ to $5 \%$ under Alternative 1. In addition, roundabouts constructed at the two ramp terminal intersections would require significant fill sections and retaining walls at the intersection corners and the off-ramp approaches due to the current roadside slopes. The proximity of the Old Highway 99 intersection to the railroad crossing will also require a shift of the intersection to the east to provide similar separation as existing conditions. <br> While it is a design constraint that influences the design, the bridge over I-5 is not anticipated to be adversely impacted by Alternative 1. | Alternative 2 would result in slightly better grades between the I-5 ramp terminal and Old Highway 99 intersections (4\%). In addition, the more compact geometry of the signalized intersections would not require the same level of fill/grading and retaining walls as Alternative 1 , particularly at the two l-5 ramp terminal intersections. Alternative 2 is not anticipated to adversely impact the bridge over I-5. | Design Constraints for Alternative 3 are consistent with Alternative 2, with the exception of the existing park and ride area located on the east side of Old Highway 99, south of Cook Road. This alternative would reduce the space available for this facility and likely require the relocation or elimination of the park and ride area. | Design Constraints for Alternative 4 are consistent with Alternative 3. |


| Factor | Alternative 1: <br> Roundabout | Alternative 2: <br> Traffic Signal | Alternative 3: <br> Traffic Signal with added NBR at Old Highway 99 N | Alternative 4: <br> Traffic Signal \& single controller |
| :---: | :---: | :---: | :---: | :---: |
| Multimodal Accommodation | This alternative would provide pedestrian crossings across the north legs of the two ramp terminal intersections. A 5 -foot sidewalk along the north side of the l-5 bridge will also be provided. Two-stage crossings across the north, west, and south legs of the Old Highway 99 roundabout would be provided with medians and Rectangular Rapid Flashing Beacons (RRFBs) provided to improve the crossing safety and experience for pedestrians while they cross multiple lanes of traffic. Due to the proximity of the railroad crossing east of Old Highway 99, a pedestrian crossing would not be provided across the east leg of the Old Highway 99 intersection. Vehicles will also be forced to slow down prior to entering the roundabout due to the design of the intersection. | The signal alternative would maintain curb ramps and signalized crossings across each leg of the Old Highway 99 intersection. install curb ramps and signalized crossings across the north legs of the l-5 ramp terminal intersections. These crossings will provide pedestrian walk phases/signals. A 5 -foot sidewalk along the north side of the l-5 bridge will also be provided. Crossing distances would increase across the west and east legs of the Old Highway 99 intersection. | Pedestrian facilities provided for Alternative 3 would be similar to Alternative 2 with the additional effect of an increased pedestrian crossing across the south leg of the Old Highway 99 intersection. | Pedestrian facilities provided for Alternative 4 would be similar to Alternative 3. |
| Safety | When evaluated as isolated intersections under normal operation, the roundabout alternative is expected to result in measurable decreases in collision frequency and severity. However, the proximity of the railroad crossing to the Old Highway 99 intersection presents a challenge to safety. In preliminary discussions with BNSF, the lack of a clear and efficient way to clear the rail crossing is a major concern. A complex gate system and/or signalization of some, or all the roundabout approaches at that intersection may be required for BNSF concurrence. This added complexity is expected to increase the risk of vehicle/train collisions. Additional discussion regarding railroad safety is included in Step 3. | The ability to interconnect the traffic signal at Old Highway 99 with the railroad crossing gate system is expected to reduce the complexity and risk of vehicle/train collisions relative to Alternative 1. This alternative provides facilities for railroad crossings that are familiar to motorists, promoting predictable behavior. In addition, the interconnect between the rail crossing and traffic signal allows preemption of the traffic signal including dedicated track clearance phases and conditional servicing of phases during train events. This allows traffic that does not conflict with the rail crossing to continue, reducing congestion and related crash types. | Consistent with Alternative 2 with respect to the ability to provide preemption of the traffic signal at the Old Highway 99 intersection. The additional northbound rightturn lane provides the added benefit of queue storage for rightturning vehicles and allows the traffic signal to continue to service northbound thru- and left-turning vehicles during a train event, further reducing congestion and related crash types. | Consistent with Alternative 2 with respect to the ability to provide preemption of the traffic signal at the Old Highway 99 intersection. The benefit of operating the northbound I-5 ramp terminal intersection and Old Highway 99 intersection with the same controller includes greater control of queues during and after a train event. This is anticipated to reduce congestion after the train event faster and minimize the amount of time the project area experiences heavy congestion and associated crash types. |
| Maintenance/ Operations | The roundabout would require less annual maintenance except for landscaping, if applicable. | The signals would require regular maintenance, service calls, and replacement of parts as needed. | Consistent with Alternative 2. | Consistent with Alternative 2. |


| Factor | Alternative 1: Roundabout | Alternative 2: <br> Traffic Signal | Alternative 3: <br> Traffic Signal with added NBR at Old Highway 99 N | Alternative 4: <br> Traffic Signal \& single controller |
| :---: | :---: | :---: | :---: | :---: |
| Implementation/ Constructability | Constructing a roundabout at the existing intersections would have significant impacts on existing traffic and would require extensive planning and traffic control. Construction would either require a full closure of the interchange or require complex staging including first constructing improvements outside of the existing roadway and then require multiple stages of traffic control to reroute traffic through a partially constructed roundabout footprint. | The traffic signal would be installed with relatively little disruption to existing traffic. The existing signal at the Old Highway 99 intersection would remain in operation, while new signal equipment is installed outside of the existing roadway. Construction of the new traffic signals at the l-5 ramp terminal intersection would only require minor disruptions to traffic. Implementation of the intersection and roadway improvements would be constructed while maintaining existing lane configurations as much as possible but may require short term closures and detours. | Consistent with Alternative 2. | Consistent with Alternative 2. |
| Cost | Alternative 1 is estimated to cost $\$ 8,000,000$. | Alternative 2 is estimated to cost $\$ 6,000,000$. Reduced relative to Alternative 1. | Alternative 3 is estimated to cost $\$ 6,300,000$, which is a slight increase relative to Alternative 2 to provide the added northbound right turn lane. | Alternative 4 is estimated to cost $\$ 6,300,000$, consistent with Alternative 3. This alternative would remove a controller but the need to extend wires would be reduced. |


| Factor | Alternative 1: Roundabout | Alternative 2: <br> Traffic Signal | Alternative 3: <br> Traffic Signal with added NBR at Old Highway 99 N | Alternative 4: <br> Traffic Signal \& single controller |
| :---: | :---: | :---: | :---: | :---: |
| Other | N/A | Note that a traffic signal warrant analysis was completed for the traffic signal alternatives (Alternatives 2-4) to confirm feasibility of the installation of a traffic signal at the existing unsignalized I-5 ramp intersections along Cook Road. | Consistent with Alternative 2. | Consistent with Alternative 2. |
|  |  | Criteria establishing warrants for installation of traffic signals is outlined in the Manual on Uniform Traffic Control Devices (MUTCD). MUTCD Chapter 4C, Section 4C.01. Warrant 1 (Eight-Hour Vehicular Volume) and Warrant 2 (FourHour Vehicular Volume) were evaluated, which are the only warrants applicable. |  |  |
|  |  | Hourly traffic volumes were developed for the analysis using the weekday PM peak hour traffic volumes for the Opening Year (2028) conditions (see Figure 4) and applying the hourly distribution from the National Cooperative Highway Research Program (NCHRP) Report 365 Travel Estimation Techniques for Urban Planning. The results of the warrant analysis show that Warrant 1 (Eight-Hour Vehicular Volume) and Warrant 2 (FourHour Vehicular Volume) are both met at both intersections under the future (2028) conditions. The traffic signal warrants are provided in Appendix C. |  |  |

## Step 3: Operational and Safety Performance Analysis

As described previously, the operations of the intersection traffic control alternatives were evaluated under an opening year of 2028 and a design year of 2045. Forecast traffic operations as well as a safety discussion are discussed in the following sections.

## Traffic Operations

The intersection level of service and vehicle queuing analysis has been summarized below to compare the No Action and Action Alternatives.

## Intersection Operations

Table 3 shows the overall intersection operations for the Action Alternatives during the weekday AM and PM peak hour under future Opening Year conditions relative to the existing No Action. The analysis considers the operations both with and without a train event. As identified above, a train event assumes a 5 minute closure for all alternatives during both the AM and PM peak hours. Detailed LOS worksheets are provided in Appendix D.

Table 3. Opening Year (2028) Weekday AM and PM Peak Hour Intersection LOS Comparisons

| Intersection | No Action |  |  | Alternative 1: <br> Roundabout |  |  | Alternative 2: <br> Traffic Signal |  | Alternative 3: <br> Traffic Signal with added NBR at Old Highway 99 N |  | Alternative 4: Traffic Signal \& Single controller |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS ${ }^{1}$ | Delay ${ }^{2}$ | $\mathrm{wM}^{3}$ | LOS | Delay | $\mathrm{v} / \mathrm{c}^{4}$ | LOS | Delay | LOS | Delay | LOS | Delay |
| AM Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |
| Without Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | C | 22 | SB | B | 12 | 0.61 | B | 17 | B | 17 | B | 19 |
| 2. I-5 NB Ramp/Cook Rd | A | 6 | NB | B | 13 | 0.79 | A | 6 | A | 6 | A | 7 |
| 3. Old Hwy 99 N/Cook Rd | B | 20 | - | B | 15 | 0.81 | B | 20 | B | 19 | C | 20 |
| With Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | D | 33 | SB | B | 12 | 0.61 | B | 18 | B | 18 | B | 19 |
| 2. I-5 NB Ramp/Cook Rd | C | 20 | NB | B | 17 | 0.79 | A | 10 | A | 10 | A | 10 |
| 3. Old Hwy 99 N/Cook Rd | C | 25 | - | B | 17 | 0.81 | C | 26 | C | 26 | C | 25 |
| PM Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |
| Without Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | B | 13 | SB | A | 8 | 0.45 | B | 17 | B | 15 | B | 17 |
| 2. I-5 NB Ramp/Cook Rd | C | 17 | NB | B | 12 | 0.73 | B | 11 | A | 10 | A | 9 |
| 3. Old Hwy 99 N/Cook Rd | B | 53 | - | B | 10 | 0.68 | C | 27 | B | 20 | B | 20 |
| With Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | C | 18 | SB | B | 12 | 0.58 | C | 23 | B | 17 | C | 20 |
| 2. I-5 NB Ramp/Cook Rd | F | 81 | NB | C | 27 | 0.97 | C | 21 | B | 18 | B | 20 |
| 3. Old Hwy 99 N/Cook Rd | F | 92 | - | B | 15 | 0.71 | D | 46 | C | 27 | C | 28 |

Note: Shading indicates the intersection operates below standard.

1. Level of Service $(A-F)$ as defined by the Highway Capacity Manual (6th Edition). Operations for non-roundabout alternatives evaluated using SimTraffic and Sidra for the roundabout alternative.
2. Average delay in seconds per vehicle.
3. Worst movement reported for unsignalized intersections. NB = northbound, $\mathrm{SB}=$ southbound
4. Volume/capacity (v/c) approach ratio.

As shown in Table 3, all study intersections operate at LOS C or better under future (2028) conditions during both the weekday AM and PM peak hours and meet the respective LOS standards without a train event.

With a train event during the peak AM peak hour under future (2028) conditions, the I-5 Southbound Ramp/Cook Road intersection is forecast to degrade to operate below standard at LOS D under the No Action condition. All study intersections with the Action Alternatives with a train event during the AM peak hour are forecast to meet the respective operational standards. During the PM peak hour with a train event, the I-5 Northbound Ramp and Old Highway 99 N and Cook Road intersections are forecast to degrade to operate below standard at LOS F under the No Action condition. All study intersections with the Action Alternatives with a train event during the PM peak hour are forecast to meet the respective operational standards.

Table 4 shows the overall intersection operations for the Action Alternatives during the weekday AM and PM peak hour under future Design Year conditions relative to the existing No Action both with and without a train event. Note that there were also adjustments to model parameters including the PHF and EF between the opening year and design year as described in the methodology section above.

Table 4. Design Year (2045) Weekday AM and PM Peak Hour Intersection LOS Comparisons

| Intersection | No Action |  |  | Alternative 1: <br> Roundabout |  |  | Alternative 2: <br> Traffic Signal |  | Alternative 3: Traffic Signal with added NBR at Old Highway 99 N |  | Alternative 4: Traffic Signal \& Single controller |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS ${ }^{1}$ | Delay ${ }^{2}$ | WM ${ }^{3}$ | LOS | Delay | $\mathrm{v} / \mathrm{c}^{4}$ | LOS | Delay | LOS | Delay | LOS | Delay |
| AM Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |
| Without Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | F | 111 | SB | B | 11 | 0.65 | C | 24 | C | 24 | C | 23 |
| 2. I-5 NB Ramp/Cook Rd | A | 8 | NB | B | 14 | 0.83 | A | 8 | A | 8 | A | 9 |
| 3. Old Hwy 99 N/Cook Rd | C | 30 | - | C | 20 | 0.87 | D | 49 | D | 50 | C | 33 |
| With Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | F | 99 | SB | B | 12 | 0.65 | C | 24 | C | 23 | C | 24 |
| 2. I-5 NB Ramp/Cook Rd | E | 36 | NB | B | 17 | 0.83 | B | 13 | B | 12 | B | 12 |
| 3. Old Hwy 99 N/Cook Rd | D | 45 | - | C | 21 | 0.87 | E | 71 | E | 65 | D | 47 |
| PM Peak Hour |  |  |  |  |  |  |  |  |  |  |  |  |
| Without Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | C | 25 | SB | A | 8 | 0.44 | C | 23 | C | 24 | D | 37 |
| 2. I-5 NB Ramp/Cook Rd | F | 150 | NB | A | 9 | 0.63 | C | 21 | B | 18 | D | 37 |
| 3. Old Hwy 99 N/Cook Rd | F | 156 | - | A | 10 | 0.62 | F | 88 | D | 37 | C | 31 |
| With Train Event |  |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | F | 152 | SB | B | 20 | 0.87 | E | 56 | D | 45 | E | 74 |
| 2. I-5 NB Ramp/Cook Rd | F | 228 | NB | C | 21 | 0.85 | D | 39 | C | 29 | E | 59 |
| 3. Old Hwy 99 N/Cook Rd | F | 180 | - | B | 16 | 0.83 | F | 153 | D | 55 | D | 41 |

Note: Shading indicates the intersection operates below standard.

1. Level of Service (A - F) as defined by the Highway Capacity Manual (6th Edition). Operations for non-roundabout alternatives evaluated using SimTraffic and Sidra for the roundabout alternative.
2. Average delay in seconds per vehicle.
3. Worst movement reported for unsignalized intersections. NB = northbound, $\mathrm{SB}=$ southbound
4. Volume/capacity (v/c) approach ratio.

As shown in Table 4, all study intersections meet the respective LOS standards without a train event under future (2045) conditions during the weekday AM peak hour with the exception of the I-5 Southbound Ramp/Cook Road intersection which is forecast to operate at LOS F under No Action conditions. With a railroad crossing during the peak AM peak hour under future (2045) conditions, the I-5 Southbound Ramp and Northbound Ramp intersections along Cook Road are forecast to degrade to operate below standard at LOS F under the No Action condition. All study intersections with the Action Alternatives with a train event during the AM peak hour are forecast to meet the respective operational standards with the exception of the Old Highway 99 N/Cook Road intersection under Alternatives 2 and 3 which are forecast to operate at LOS E.

During the PM peak hour, the intersections of l-5 Northbound Ramps and Old Highway 99 N along Cook Road are forecast to operate at LOS F under the No Action conditions, operating below standard. All study intersections under only Alternatives 1 and 3 during the PM peak hour under future (2045) conditions are forecast to meet the operational standards. With a train event, all study intersections during the PM peak hour No Action and Alternative 2 conditions are forecast to operate at LOS F, below the operational standard.

## Vehicle Queues

The 95th-percentile vehicle queues for the No Action and Action Alternatives during the weekday AM and PM peak hours are summarized in Tables 5 and 6 under the Opening and Design Years, respectively. The queues are reviewed both with and without the train event. The 95th-percentile queues represent the vehicle queue lengths that would only be exceeded 5 percent of the time during the peak hour. The detailed queueing worksheets are included in Appendix D.

Under 2028 conditions, the 95th percentile queues are generally accommodated within the available storage during both the weekday AM and PM peak hours, without a train event under all alternatives (No Action and Action). With a train event, the greatest queues occur westbound at the Old Highway 99 N/Cook Road intersection in both the AM and PM peak hours, as well as the northbound approach at both the I-5 Northbound Ramps and Old Highway 99 N intersections along Cook Road under the No Action condition. With the traffic signal Action Alternatives, the queues are forecast to be similar or less than the No Action condition whereas the roundabout alternative results in increases in queues in the westbound approach along Cook Road.

Similar to 2028 conditions, under future (2045) conditions, the 95th percentile queues are generally accommodated within the available storage during both the weekday AM and PM peak hours, without a train event under all alternatives (No Action and Action). With a train event, the greatest queues occur westbound and northbound at the Old Highway 99 N/Cook Road intersection, northbound at the I-5 Northbound Ramps/Cook Road, and southbound at the l-5 Southbound Ramps/Cook Road intersection in both the AM and PM peak hours under the No Action condition. With the traffic signal Action Alternatives, the queues are forecast to be similar or less than the No Action condition whereas the roundabout alternative results in increases in queues in the westbound approach along Cook Road.

The reduced queueing of the signalized alternatives with the train event relative to other alternatives, particularly Alternatives 3 and 4 which add a northbound right-turn lane at the Old Highway 99 N/Cook Road intersection, is associated with the added capacity and separation of the movements, allowing for continued processing of vehicles during the train event unlike the other alternatives.

Table 5. Opening Year (2028) Weekday AM and PM Peak Hour 95th-Percentile Vehicle Queue Comparisons

|  |  | 95th Percentile Queue ${ }^{2}$ (ft) Without Train Event |  |  |  |  | 95th Percentile Queue ${ }^{2}$ (ft) With Train Event |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | Available Storage ${ }^{1}$ (ft) | No Action | Alt 1 : RAB | Alt 2: <br> Traffic Signal | Alt 3: Traffic Signal + NBR | Alt 4 : <br> Traffic Signal + Single Controller | No Action | Alt 1: RAB | Alt 2: <br> Traffic Signal | Alt 3: <br> Traffic Signal + NBR | Alt 4: <br> Traffic Signal + Single Controller |

## AM Peak Hour

1. I-5 SB Ramp/Cook Rd

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Eastbound | $>1,000$ | 0 | 15 | 65 | 60 | 60 | 40 | 15 | 60 | 60 | 65 |
| Westbound | 470 | 130 | 0 | 320 | 330 | 345 | 140 | 0 | 340 | 325 | 345 |
| $\quad$ Southbound | 875 | 380 | 25 | 255 | 240 | 280 | 570 | 25 | 260 | 255 | 280 |
| 2. I-5 NB Ramp/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound | 470 | 80 | 0 | 115 | 125 | 150 | 290 | 180 | 235 | 215 | 220 |
| Westbound | 225 | 20 | 145 | 210 | 195 | 215 | 5 | 145 | 220 | 220 | 205 |
| Northbound Through/Right | 1,180 | 180 | 15 | 95 | 90 | 95 | 625 | 140 | 160 | 155 | 135 |
| $\quad$ Northbound Right | 500 | - | - | 70 | 70 | 70 | - | - | 130 | 135 | 105 |
| 3. Old Hwy 99 N/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound Left | 150 | 140 | 15 | 125 | 120 | 130 | 150 | $\mathbf{2 2 5}$ | 125 | 140 | 145 |
| Eastbound Through | 225 | $\mathbf{3 1 0}$ | 15 | 195 | 200 | 200 | $\mathbf{3 3 5}$ | 225 | $\mathbf{2 4 5}$ | $\mathbf{2 5 0}$ | $\mathbf{2 4 5}$ |
| Westbound Left | 275 | 285 | 10 | 220 | 215 | 265 | $\mathbf{3 3 5}$ | $\mathbf{3 2 5}$ | $\mathbf{2 9 0}$ | $\mathbf{3 1 0}$ | $\mathbf{3 0 0}$ |
| Westbound Through | $>8,000^{3}$ | 730 | 110 | 540 | 540 | 665 | 2,295 | 6,030 | 1,810 | 2,305 | 2,070 |
| Northbound Left | 100 | $\mathbf{1 1 5}$ | 10 | $\mathbf{1 2 0}$ | $\mathbf{1 2 0}$ | $\mathbf{1 7 5}$ | $\mathbf{1 2 0}$ | 10 | $\mathbf{1 2 0}$ | $\mathbf{1 2 0}$ | $\mathbf{1 7 0}$ |
| Northbound Through | $>1,000$ | 215 | 10 | 285 | 205 | 185 | 300 | 195 | 310 | 305 | 180 |
| Northbound Right | 300 | - | 10 | - | 95 | 75 | - | 195 | - | 140 | 95 |
| Southbound Left | 200 | 105 | 10 | 105 | 110 | 115 | 110 | $\mathbf{2 1 0}$ | 115 | 120 | 115 |
| Southbound Through | $>2,000$ | 260 | 45 | 290 | 305 | 335 | 345 | 45 | 405 | 425 | 430 |
| Southbound Right | 100 | $\mathbf{1 4 0}$ | 45 | $\mathbf{1 4 0}$ | $\mathbf{1 4 0}$ | 145 | $\mathbf{1 4 0}$ | 45 | $\mathbf{1 4 5}$ | $\mathbf{1 4 5}$ | $\mathbf{1 4 5}$ |

## PM Peak Hour

1. I-5 SB Ramp/Cook Rd

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Eastbound | $>1,000$ | 5 | 15 | 80 | 85 | 95 | 95 | 25 | 150 | 95 | 130 |
| Westbound | 470 | 120 | 0 | 260 | 230 | 255 | 120 | 0 | 310 | 265 | 290 |
| $\quad$ Southbound | 875 | 265 | 15 | 225 | 210 | 235 | 335 | 20 | 290 | 230 | 245 |
| 2. I-5 NB Ramp/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound | 470 | 225 | 0 | 300 | 255 | 230 | 405 | 340 | 470 | 410 | 420 |
| Westbound | 225 | 10 | 90 | $\mathbf{2 5 0}$ | $\mathbf{2 5 5}$ | 200 | 50 | 90 | $\mathbf{2 8 5}$ | $\mathbf{2 4 5}$ | $\mathbf{2 3 0}$ |
| $\quad$ Northbound Through/Right | 1,180 | 690 | 35 | 135 | 130 | 150 | $\mathbf{1 , 9 8 0}$ | 370 | 215 | 200 | 265 |
| Northbound Right | 500 | - | - | 95 | 85 | 110 | - | - | 170 | 160 | 230 |
| 3. Old Hwy 99 N/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound Left | 150 | $\mathbf{1 8 0}$ | 20 | $\mathbf{1 6 5}$ | 150 | $\mathbf{1 6 5}$ | $\mathbf{1 8 0}$ | $\mathbf{2 2 5}$ | $\mathbf{1 7 0}$ | $\mathbf{1 7 0}$ | $\mathbf{1 7 5}$ |
| Eastbound Through | 225 | $\mathbf{3 3 5}$ | 20 | $\mathbf{2 7 0}$ | 215 | $\mathbf{2 6 5}$ | $\mathbf{3 2 0}$ | 225 | $\mathbf{3 0 5}$ | $\mathbf{2 7 5}$ | $\mathbf{3 0 5}$ |
| Westbound Left | 275 | 235 | 5 | 210 | 195 | 180 | 255 | 165 | 265 | 245 | 225 |
| Westbound Through | $>8,000^{\mathbf{3}}$ | 835 | 90 | 725 | 540 | 620 | 2,385 | 4,415 | 3,265 | 1,840 | 1,845 |
| Northbound Left | 100 | $\mathbf{1 2 5}$ | 10 | $\mathbf{1 1 5}$ | $\mathbf{1 2 0}$ | $\mathbf{1 5 5}$ | $\mathbf{1 3 0}$ | 10 | $\mathbf{1 3 0}$ | $\mathbf{1 2 5}$ | $\mathbf{1 6 5}$ |
| Northbound Through | $>1,000$ | $\mathbf{1 , 4 3 0}$ | 35 | 525 | 340 | 250 | $\mathbf{2 , 3 1 0}$ | 465 | $\mathbf{1 , 0 9 0}$ | 475 | 330 |
| Northbound Right | 300 | - | 35 | - | 190 | 85 | - | $\mathbf{4 6 5}$ | - | 225 | 205 |
| Southbound Left | 200 | 100 | 5 | 85 | 110 | 95 | 110 | $\mathbf{2 6 0}$ | 100 | 105 | 100 |
| Southbound Through | $>2,000$ | 155 | 20 | 155 | 200 | 185 | 210 | 20 | 220 | 230 | 300 |
| Southbound Right | 100 | $\mathbf{1 2 0}$ | 20 | $\mathbf{1 1 5}$ | $\mathbf{1 2 5}$ | $\mathbf{1 2 5}$ | $\mathbf{1 2 5}$ | 20 | $\mathbf{1 2 0}$ | $\mathbf{1 3 0}$ | $\mathbf{1 2 0}$ |

Note: RAB = roundabout. Bold indicates the queue exceeds the available storage AND shading indicates those locations that also exceed the respective No Action queue.

1. The storage length represents the available lane length for cars to queue, rounded to the nearest 25 feet.
2. 95th Percentile queues are derived from SimTraffic and Sidra, rounded to the nearest 25 feet
3. 50 ft to railroad crossing or 445 ft to Green Road; however, queuing would continue through side street stop-controlled intersections.

Table 6. Design Year (2045) Weekday AM and PM Peak Hour 95th-Percentile Vehicle Queue Comparisons

|  |  | 95th Percentile Queue ${ }^{2}$ (ft) Without Train Event |  |  |  |  | 95th Percentile Queue ${ }^{2}$ (ft) With Train Event |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | Available Storage ${ }^{1}$ (ft) | $\begin{gathered} \text { No } \\ \text { Action } \end{gathered}$ | Alt 1: RAB | Alt 2: <br> Traffic <br> Signal | Alt 3: <br> Traffic <br> Signal <br> + NBR | Alt 4: <br> Traffic Signal <br> + Single <br> Controller | No Action | Alt 1: RAB | Alt 2: <br> Traffic <br> Signal | Alt 3: <br> Traffic <br> Signal + <br> NBR | Alt 4: <br> Traffic Signal + Single Controller |

AM Peak Hour

1. I-5 SB Ramp/Cook Rd

| Eastbound | $>1,000$ | 5 | 15 | 85 | 75 | 75 | 90 | 15 | 85 | 75 | 80 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\quad$ Westbound | 470 | 150 | 0 | 400 | 420 | 425 | 145 | 0 | 440 | 410 | 420 |
| $\quad$ Southbound | 875 | $\mathbf{1 , 8 2 0}$ | 25 | 370 | 350 | 340 | $\mathbf{1 , 5 0 0}$ | 30 | 345 | 320 | 350 |
| 2. I-5 NB Ramp/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound | 470 | 120 | 0 | 205 | 245 | 215 | 370 | 180 | 320 | 325 | 315 |
| Westbound | 225 | 10 | 170 | $\mathbf{2 5 0}$ | $\mathbf{2 5 5}$ | $\mathbf{2 3 5}$ | 5 | 170 | $\mathbf{2 7 5}$ | $\mathbf{2 7 0}$ | $\mathbf{2 4 5}$ |
| Northbound Through/Right | 1,180 | 230 | 15 | 100 | 115 | 115 | 920 | 135 | 185 | 170 | 150 |
| $\quad$ Northbound Right | 500 | - | - | 70 | 70 | 85 | - | - | 150 | 145 | 120 |
| 3. Old Hwy 99 N/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound Left | 150 | $\mathbf{1 7 0}$ | 20 | 145 | $\mathbf{1 6 5}$ | $\mathbf{1 6 0}$ | $\mathbf{1 7 5}$ | $\mathbf{2 2 5}$ | $\mathbf{1 6 5}$ | $\mathbf{1 7 0}$ | $\mathbf{1 6 0}$ |
| Eastbound Through | 225 | $\mathbf{3 3 0}$ | 20 | $\mathbf{2 4 5}$ | $\mathbf{2 6 0}$ | $\mathbf{2 4 0}$ | $\mathbf{3 3 5}$ | 225 | $\mathbf{2 8 5}$ | $\mathbf{2 8 0}$ | $\mathbf{2 5 5}$ |
| Westbound Left | 275 | $\mathbf{3 9 0}$ | 15 | $\mathbf{3 7 0}$ | $\mathbf{3 7 5}$ | $\mathbf{3 6 0}$ | $\mathbf{3 7 5}$ | $\mathbf{5 2 5}$ | 200 | $\mathbf{3 6 5}$ | $\mathbf{3 7 0}$ |
| Westbound Through | $>8,000^{3}$ | 3,400 | 115 | 875 | 860 | 2,905 | 5,220 | 7,625 | 4,485 | 5,110 | 4,860 |
| Northbound Left | 100 | $\mathbf{1 2 0}$ | 5 | 120 | 120 | $\mathbf{1 8 5}$ | $\mathbf{1 2 0}$ | $\mathbf{5}$ | 120 | 120 | $\mathbf{1 9 0}$ |
| Northbound Through | $>1,000$ | 250 | 10 | 530 | 395 | 220 | 420 | 190 | 645 | 600 | 270 |
| Northbound Right | 300 | - | - | - | 185 | 85 | - | - | - | 195 | 140 |
| Southbound Left | 200 | 120 | 10 | 120 | 115 | 115 | 120 | $\mathbf{2 1 5}$ | 120 | 125 | 125 |
| Southbound Through | $>2,000$ | 630 | 115 | 1,380 | 1,330 | 830 | 1,120 | 110 | $\mathbf{1 , 7 3 5}$ | $\mathbf{1 , 6 0 5}$ | 1,335 |
| Southbound Right | 100 | $\mathbf{1 5 0}$ | $\mathbf{1 1 5}$ | $\mathbf{1 5 0}$ | $\mathbf{1 4 5}$ | $\mathbf{1 4 5}$ | $\mathbf{1 5 5}$ | $\mathbf{1 1 0}$ | $\mathbf{1 5 0}$ | $\mathbf{1 5 0}$ | $\mathbf{1 5 5}$ |

## PM Peak Hour

1. I-5 SB Ramp/Cook Rd

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Eastbound | $>1,000$ | 30 | 30 | 190 | 220 | 360 | 455 | 80 | 505 | 410 | 560 |
| $\quad$ Westbound | 470 | 170 | 0 | 330 | 390 | 420 | 235 | 0 | 375 | 405 | $\mathbf{5 2 0}$ |
| $\quad$ Southbound | 875 | 470 | 15 | 305 | 265 | 375 | $\mathbf{1 , 9 3 0}$ | 15 | 630 | 575 | 770 |
| 2. I-5 NB Ramp/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound | 470 | 455 | 0 | $\mathbf{5 1 5}$ | $\mathbf{5 0 5}$ | $\mathbf{5 7 5}$ | $\mathbf{5 9 0}$ | $\mathbf{5 0 0}$ | $\mathbf{6 1 5}$ | $\mathbf{6 0 0}$ | $\mathbf{6 3 0}$ |
| Westbound | 225 | 15 | 60 | $\mathbf{3 2 0}$ | $\mathbf{3 1 0}$ | $\mathbf{2 4 0}$ | 20 | 60 | $\mathbf{3 4 0}$ | $\mathbf{3 3 0}$ | $\mathbf{2 8 0}$ |
| $\quad$ Northbound Through/Right | 1,180 | $\mathbf{3 , 1 1 0}$ | 30 | 185 | 160 | 490 | $\mathbf{3 , 2 3 5}$ | 370 | 265 | 225 | 700 |
| $\quad$ Northbound Right | 500 | - | - | 120 | 115 | 450 | - | - | 215 | 175 | $\mathbf{6 6 5}$ |
| 3. Old Hwy 99 N/Cook Rd |  |  |  |  |  |  |  |  |  |  |  |
| $\quad$ Eastbound Left | 150 | $\mathbf{1 7 5}$ | 20 | $\mathbf{1 7 5}$ | $\mathbf{1 8 0}$ | $\mathbf{1 8 0}$ | $\mathbf{1 9 5}$ | $\mathbf{2 2 5}$ | $\mathbf{1 9 0}$ | $\mathbf{1 9 0}$ | $\mathbf{1 9 5}$ |
| Eastbound Through | 225 | $\mathbf{3 1 0}$ | 20 | $\mathbf{3 1 5}$ | $\mathbf{3 1 0}$ | $\mathbf{3 3 5}$ | $\mathbf{3 1 0}$ | 225 | $\mathbf{3 3 0}$ | $\mathbf{3 3 0}$ | $\mathbf{3 3 5}$ |
| Westbound Left | 275 | $\mathbf{2 9 0}$ | 5 | $\mathbf{2 9 5}$ | $\mathbf{2 8 0}$ | 245 | $\mathbf{3 0 0}$ | 165 | 270 | $\mathbf{3 1 0}$ | 265 |
| Westbound Through | $>8,000^{3}$ | 1,790 | 90 | 760 | 870 | 980 | 3,400 | 4,475 | 4,950 | 4,100 | 2,915 |
| Northbound Left | 100 | 130 | 5 | 125 | 125 | 200 | 120 | $\mathbf{5}$ | 120 | 120 | 205 |
| Northbound Through | $>1,000$ | $\mathbf{3 , 2 8 0}$ | 50 | $\mathbf{2 , 3 3 0}$ | 905 | 445 | $\mathbf{3 , 0 5 0}$ | 575 | $\mathbf{3 , 1 1 0}$ | $\mathbf{1 , 3 1 0}$ | 690 |
| Northbound Right | 300 | - | - | - | 290 | 260 | - | - | - | 300 | $\mathbf{3 3 0}$ |
| Southbound Left | 200 | 110 | 5 | 100 | 110 | 105 | 110 | $\mathbf{2 8 5}$ | 105 | 120 | 115 |
| Southbound Through | $>2,000$ | 195 | 20 | 185 | 210 | 195 | 335 | 20 | 365 | 375 | 375 |
| Southbound Right | 100 | $\mathbf{1 3 0}$ | 20 | $\mathbf{1 2 5}$ | $\mathbf{1 3 0}$ | $\mathbf{1 2 5}$ | $\mathbf{1 3 0}$ | 20 | $\mathbf{1 2 5}$ | $\mathbf{1 3 5}$ | $\mathbf{1 3 5}$ |

Note: RAB = roundabout. Bold indicates the queue exceeds the available storage AND shading indicates those locations that also exceed the respective No Action queue.

1. The storage length represents the available lane length for cars to queue, rounded to the nearest 25 feet.
2. 95th Percentile queues are derived from SimTraffic and Sidra, rounded to the nearest 25 feet
3.50 ft to railroad crossing or 445 ft to Green Road; however, queuing would continue through side street stop-controlled intersections.

## Safety Performance Analysis

This section summarizes the five-year crash history at the study intersections and provides a vehicular safety comparison for the alternatives.

## Analysis of Existing Intersection Crashes

Crash records over the most recent complete 5-year period were reviewed at the northbound and southbound I-5 ramps intersections along Cook Road to identify potential safety issues within the vicinity of the study area. Collisions along the I-5 northbound and southbound ramp segments as well as the I-5 mainlines ${ }^{3}$ were also reviewed to identify safety issues related to queueing during train events. Reported crash data was provided by WSDOT for the period of January 1, 2018 to December 31, 2022 for the study area. Table 7 provides a summary of the total number, type, and severity of the reported collisions.

Table 7. Five-Year Collision Summary (By Collision Type) - 2018 to 2022

| Location | Collision Type |  |  |  |  |  | Severity |  |  | Total | Average Annual |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Approach Turn | Rear End | Ped/ Bike | Angle | Sideswipe | Fixed Object | PDO ${ }^{1}$ | Injury | Fatality |  |  |
| Intersections |  |  |  |  |  |  |  |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | 1 | 4 | 0 | 4 | 0 | 5 | 12 | 2 | 0 | 14 | 2.8 |
| 2. I-5 NB Ramp/Cook Rd | 0 | 4 | 0 | 7 | 0 | 3 | 10 | 4 | 0 | 14 | 2.8 |
| 3. Old Hwy 99 N/Cook Rd | 6 | 7 | 0 | 7 | 4 | 0 | 19 | 5 | 0 | 24 | 4.8 |

## Roadway Segments

l-5 Southbound

| Ramp (Cook Rd to I-5 Mainline) | 0 | 4 | 0 | 0 | 0 | 1 | 3 | 2 | 0 | 5 | 1.0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Total Mainline (MP 232.98-233.73) | 0 | 0 | 0 | 0 | 1 | 8 | 9 | 0 | 0 | 9 | 1.8 |
| Associated with Train Event ${ }^{2}$ | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| I-5 Northbound |  |  |  |  |  |  |  |  |  |  |  |
| Ramp (Cook Rd to I-5 Mainline) | 0 | 10 | 0 | 0 | 0 | 1 | 8 | 3 | 0 | 11 | 2.2 |
| Total Mainline (MP 231.95-232.70) | 0 | 5 | 0 | 0 | 1 | 8 | 13 | 1 | 0 | 14 | 2.8 |
| Associated with Train Event ${ }^{2}$ | 0 | 5 | 0 | 0 | 0 | 0 | 4 | 1 | 0 | 5 | 1.0 |

Source: WSDOT, 2023.
Note: Under 23 U.S. Code § 409 and 23 U.S. Code § 148, safety data, reports, surveys, schedules, lists compiled or collected for the purpose of identifying, evaluating, or planning the safety enhancement of potential crash sites, hazardous roadway conditions, or railway-highway crossings are not subject to discovery or admitted into evidence in a Federal or State court proceeding or considered for other purposes in any action for damages arising from any occurrence at a location mentioned or addressed in such reports, surveys, schedules, lists, or data.

1. PDO = property damage only (i.e. no apparent injury)
2. Train event assumed collision notes slowing or stopped vehicle involved. Note collisions only include respective direction of travel (e.g. northbound only includes vehicles traveling south to north).

The collision history review above shows that there were on average 3 collisions reported per year at the unsignalized southbound and northbound l-5 ramp intersections along Cook Road, respectively. The signalized Old Highway 99 N/Cook Road intersection in the study area showed an average annual of 5 collisions reported per year. Of the total collisions in the study area, the majority were rear-end and entering at an angle collisions and 80 percent of the overall reported collisions resulted in property damage only.

None of the reported collisions in the study area during the 5-year period reviewed (2018-2022) resulted in fatality nor were any reported that involved either a pedestrian or bicyclist. However, following the review period (June 2023) there has been one reported collision involving a bicyclist which occurred at the Old Highway 99 N/Cook Road intersection. The collision was a northbound left-turning vehicle striking a southbound cyclist and resulted in an injury. The cause of the collision was likely driver inattention. The

[^2]alternatives would provide for modifications at all intersections including the Old Highway 99 N/Cook Road intersection where the recent collision involving a cyclist occurred.

In addition to the study intersections, Northbound I-5 had an average of 2.2 collisions per year along the ramp with an additional 1.0 collisions along the mainline associated with slowing or stopped traffic within the vicinity of the Cook Ramp interchange with nearly all of these collisions being rear-end collisions. Rear-end collision are typical of congested conditions and queueing.

Southbound I-5 had an average of 1.0 collisions per year along the ramp with no collisions identified on the mainline associated with slowing or stopped traffic within the vicinity of the Cook Ramp interchange.

In addition to reviewing the reported collisions, a discussion with a Washington State Patrol (WSP) sergeant ${ }^{4}$ provided additional insight into existing safety conditions at the interchange based on daily observations in the vicinity. The interchange was identified to be along a known high collision area such that patrols are there regularly. There are daily weekday occurrences of eastbound queuing that occurs along Cook Road and results in queues extending onto the off-ramps, consistently on the northbound ramp but can also extend onto the southbound ramp as well. Additionally, it is estimated that 2-3 days a week, the northbound queues will extend onto the l-5 northbound mainline. Given the existing stopcontrolled traffic control at the ramp intersections, queuing along Cook Road does not provide needed gaps in the flow of traffic to clear the queues. Train events only exacerbate these challenges. The collision summary above supports these observations, with northbound ramp and mainline rear-end collisions (commonly congestion related collisions) identified to be an average of 3 collisions per year.

Additional discussion regarding the different alternatives and the multimodal considerations are provided below.

## Expected and Predicted Intersection Crashes Methodology

Analysis of the traffic safety among the two options was coordinated with WSDOT and involves the usage of HSM spreadsheets ${ }^{5}$ and Crash Modification Factors (CMFs). Note that this analysis does not consider the train event. Additional discussion of safety with a train event is provided in a subsequent section (Railroad Safety and Operations).

HSM spreadsheets were developed by the TRB Highway Safety Performance Committee and are used to calculate expected and predicted crash rates by severity by inputting intersection parameters such as AADT by approach, number of lanes, lighting availability, and other parameters. HSM spreadsheets were used to analyze the existing signalized study intersection as well as the traffic signal.

For the roundabout intersection control option, Crash Modification Factors Clearinghouse ${ }^{6}$ was referenced for a crash modification factor (CMF) as directed by WSDOT to apply to the predicted crash frequency of the signalized option to estimate crash reductions when installing a roundabout. The CMF WSDOT Reference numbers and details used to estimate safety benefits are summarized below:

- I-5 SB Ramp/Cook Road and I-5 NB Ramp/Cook Road Intersections:
- WSDOT Reference \#4931 - Convert Unsignalized Intersection to Roundabout
- Crash Type $=$ Injury and Fatal
- CMF Value $=0.65$
- Old Highway 99 Road/Cook Road Intersection:
- WSDOT Reference \#4256 (Convert Signal to Roundabout, AADT exceeds 18,000)
- Crash Type = All Collision Types - all severities

[^3]- $\quad$ CMF Value $=1.0$ (AADT exceeds 18,000$)$


## Comparison of Crash Reduction

This section summarizes the crash reduction for each alternative based on the predicted fatal and injury crash frequency and crash severity distributions. Full details including calculations and severity distributions are shown in Appendix E. Table 8 below summarizes the findings completed for the Opening Year future condition.

The methodology to estimate the crash reduction for each alternative was consistent with previous similar efforts that were coordinated with WSDOT. This methodology does not consider interactions with the railroad crossing located in close proximity to the Old Highway 99 N/Cook Road intersection. Therefore, this crash reduction analysis reflects conditions without a train event.

Table 8. Injury and Fatal Crash Reduction Summary (Without a train event)

|  | Opening Year 2028 |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
|  | No Action ${ }^{1,2}$ | Alt 1: <br> Roundabout $^{3}$ | Alt 2: <br> Traffic Signal $^{2}$ | Alt 3 \& Alt 4: <br> Traffic Signal with NBR |
| Predicted Crash Frequency for Alternative (Injury and Fatal) |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | 0.6 | 0.4 | 0.4 | 0.4 |
| 2. I-5 NB Ramp/Cook Rd | 0.9 | 0.6 | 0.7 | 0.7 |
| 3. Old Hwy 99 N/Cook Rd | 0.7 | 0.7 | 0.7 | 0.7 |
| Annual Crash Reduction (relative to No Action) |  |  |  |  |
| 1. I-5 SB Ramp/Cook Rd | - | -0.2 | -0.2 | -0.2 |
| 2. I-5 NB Ramp/Cook Rd | - | -0.3 | -0.2 | -0.2 |
| 3. Old Hwy 99 N/Cook Rd | - | 0 | 0 | 0 |

Note: Collisions reflect predicted injury and fatal crashes per year.

1. Existing assuming the existing intersection channelization.
2. Based on a combined CMF calculated per the HSM spreadsheet.
3. Predicted Crash Frequency estimated for I-5 SB and NB Ramps by No Action Crash Factor x CMF 0.65 and for the Old Hwy 99 by No Action Crash Factor x CMF 1.0

As shown in Table 8, under conditions without a train event, the Action Alternatives are anticipated to reduce the annual injury and fatal crash types at the two ramp intersections relative to the No Action condition which would change the traffic control from the existing side street stop controlled to either a roundabout or traffic signal. No change in annual injury and fatal crash types is predicted at the Old Highway $99 \mathrm{~N} / C o o k$ Road intersection. Overall, the alternatives result in similar improvement with the roundabout alternative (Alternative 1) having a slightly higher safety improvement at the I-5 Northbound Ramp/Cook Road intersection relative to the signalized alternatives.

In addition to the crash reduction analysis above, it is important to reiterate a project objective is to alleviate any queueing impacts to the l-5 mainline that occurs today and results in safety concerns including identified collisions as shown and discussed above. The signalized alternatives (Alternatives 24) allow for signal timing prioritization to minimize northbound queueing which is not a feature of the roundabout alternative (Alternative 1).

Additional discussion of safety with a train event is provided in a subsequent section (Railroad Safety and Operations).

## Multimodal Safety and Operations

With the roundabout alternative, drivers would be expected to yield to pedestrians and bicyclists at the crossings. Bicyclists could choose to either ramp up to the sidewalk (where provided) and travel through
the roundabout as a pedestrian or merge into the travel lane and function as a vehicle when entering and exiting the roundabout. Additionally, drivers must slow down prior to entering the roundabout due to the design of the intersection.

For the traffic signal alternative, signalized crossings would be maintained at the Old Highway 99 N/Cook Road intersection with crossing distances ranging between 3 and 4 lanes. Additionally, curb ramps and signalized crossings would be installed across the north legs of the I-5 ramp terminal intersections. These crossings will provide pedestrian walk phases/signals. Finally, a 5 -foot sidewalk would be added along the north side of the l-5 bridge.

For the roundabout alternative, pedestrians would be required to cross multiple lanes. A Rectangular Rapid Flashing Beacon (RRFB) system would be installed at these crossings to improve the crossing safety and experience for pedestrians as they cross multiple lanes of traffic. The RRFB systems would likely operate as a single system for each crossing so the RRFB would flash for the entire time for a pedestrian to cross from one side of the roadway to the other.

## Railroad Safety and Operations

When evaluated as isolated intersections under normal operation, each alternative is expected to result in measurable decreases in collision frequency and severity. However, the proximity of the railroad crossing to the Old Highway 99 intersection presents a challenge to safety. In preliminary discussions with BNSF, the lack of a clear and efficient way to clear the rail crossing under Alternative 1 is a major concern. In order to ensure no vehicles enter the roundabout during a train event, it is anticipated that a gate system and/or signalization of some, or all of the roundabout approaches at the intersection would be required. Also, extended red times are anticipated also to be necessary to provide sufficient time for all vehicles to circulate through the roundabout prior to gate closures. This added closure time to accommodate roundabout circulation would result in added delay and queueing than reflected in the operational analysis above which did not include added signalization of the roundabout.

Facilities provided for railroad crossings under signalized alternatives are familiar to motorists, promoting predictable behavior. In addition, the interconnect between the rail crossing and traffic signal allows preemption of the traffic signal including dedicated track clearance phases and conditional servicing of phases during train events. This allows traffic that does not conflict with the rail crossing to continue, reducing congestion and related crash types. Further, the interconnect to the signal system provides greater control of queues after a train event. This is anticipated to reduce congestion after the train event faster and minimize the amount of time the project area experiences heavy congestion and associated crash types.

## Step 4: Alternative Evaluation

All Action Alternatives will improve operations both with and without a train event. The roundabout alternative (Alternative 1) had the lowest overall delay at the study intersections both with and without a train event; however, Alternative 1 is forecast to have greater queuing following a train event compared to the signalized alternatives. Of the signalized alternatives, Alternative 3 is forecast to generally meet the operational standards with one exception which occurs under 2045 PM peak hour train event conditions at the I-5 Southbound Ramp/Cook Road intersection degrading the LOS D.

Although roundabouts generally include safety benefits relative to signalized intersections, the safety reduction analysis shows the signalized alternatives to be approximately consistent with the roundabout alternative, when considered as isolated intersections. However, the proximity of the at-grade railroad crossing to the Old Highway 99N/Cook Road intersection would most likely require a more complex gate and control system for Alternative 1 to ensure traffic can be cleared from the crossing prior to a train event. Also, signalization of the intersections allows for prioritization of the ramps to minimize queueing impacts to the I-5 mainline which occur today.

Additionally, as noted previously, the improvements are an interim condition until such time a grade separated railroad crossing is installed. Given the temporary nature of the improvements, the less invasive signalization (Alternatives 2-4) are better interim improvements.

## Step 5: Selection

The recommendation for the intersection improvements is the Alternative 3 traffic signal alternative. This alternative would include the following:

- Install traffic signals at the 2 ramp intersections,
- Add a northbound right turn lane at the l-5 northbound ramp intersection,
- Widen Cook Road to include an additional eastbound through lane east of the l-5 northbound ramp intersection to east of Green Road, and
- Add a northbound right-turn lane at the Old Highway 99 N intersection.

Alternative 3 meets the operational standards at all study intersections during both future opening and design year conditions in the AM and PM peak hours as well as both with and without a train event with the exception of the l-5 Southbound Ramp/Cook Road intersection operating at LOS D with a train event during the PM peak hour design year condition. The 95th percentile queueing is also improved with this alternative relative to the other alternatives and signalization of the intersections allows for prioritization of the ramps to minimize impacts to the l-5 mainline. Additionally, this alternative is consistent with the grant funding application, is compatible with the train crossing (acceptable by BNSF Railroad operators), is lower cost, lower ROW impact, and eliminates the design constraints that the roundabout alternative would otherwise include with similar anticipated safety reduction relative to the No Action condition.

Finally, as this is an interim condition with long-term plans to include grade separation of the railroad crossing, the selected alternative is a reasonable option for providing lower cost improvements while meeting the project needs as described above.

## Appendix A: Intersection Alternatives Concept Illustrations




Cook Rd \& I-5 Southbound Ramps - Roundabout Horizontal Layout


Cook Rd \& I-5 Northbound Ramps - Roundabout Horizontal Layout




Alternative 3 - Signalized Intersections with NB Right-Turn at Old Hwy 99

## Appendix B: LOS Definitions

## Highway Capacity Manual 2010/6th Edition

Signalized intersection level of service (LOS) is defined in terms of a weighted average control delay for the entire intersection. Control delay quantifies the increase in travel time that a vehicle experiences due to the traffic signal control as well as provides a surrogate measure for driver discomfort and fuel consumption. Signalized intersection LOS is stated in terms of average control delay per vehicle (in seconds) during a specified time period (e.g., weekday PM peak hour). Control delay is a complex measure based on many variables, including signal phasing and coordination (i.e., progression of movements through the intersection and along the corridor), signal cycle length, and traffic volumes with respect to intersection capacity and resulting queues. Table 1 summarizes the LOS criteria for signalized intersections, as described in the Highway Capacity Manual 2010 and 6th Edition (Transportation Research Board, 2010 and 2016, respectively).

Table 1. Level of Service Criteria for Signalized Intersections

| Level of Service | Average Control Delay <br> (seconds/vehicle) | General Description |
| :---: | :---: | :--- |
| A | $\leq 10$ | Free Flow |
| B | $>10-20$ | Stable Flow (slight delays) |
| C | $>20-35$ | Stable flow (acceptable delays) |
| D | $>35-55$ | Approaching unstable flow (tolerable delay, occasionally wait through more <br> than one signal cycle before proceeding) |
| E | $>55-80$ | Unstable flow (intolerable delay) |
| F $^{1}$ | $>80$ | Forced flow (congested and queues fail to clear) |
| Source: Highway Capacity Manual 2010 and 6th Edition, Transportation Research Board, 2010 and 2016, respectively. <br> 1. If the volume-to-capacity (v/c) ratio for a lane group exceeds 1.0 LOS F is assigned to the individual lane group. LOS for overall approach or <br> intersection is determined solely by the control delay. |  |  |

Unsignalized intersection LOS criteria can be further reduced into two intersection types: all-way stop and two-way stop control. All-way stop control intersection LOS is expressed in terms of the weighted average control delay of the overall intersection or by approach. Two-way stop-controlled intersection LOS is defined in terms of the average control delay for each minor-street movement (or shared movement) as well as major-street left-turns. This approach is because major-street through vehicles are assumed to experience zero delay, a weighted average of all movements results in very low overall average delay, and this calculated low delay could mask deficiencies of minor movements. Table 2 shows LOS criteria for unsignalized intersections.

Table 2. Level of Service Criteria for Unsignalized Intersections

| Level of Service | Average Control Delay (seconds/vehicle) |
| :---: | :---: |
| A | $0-10$ |
| B | $>10-15$ |
| C | $>15-25$ |
| E | $>25-35$ |
| F $^{1}$ | $>35-50$ |

Source: Highway Capacity Manual 2010 and 6th Edition, Transportation Research Board, 2010 and 2016, respectively.

1. If the volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio exceeds 1.0 , LOS $F$ is assigned an individual lane group for all unsignalized intersections, or minor street approach at two-way stop-controlled intersections. Overall intersection LOS is determined solely by control delay.

## Appendix C: Signal Warrants



| Warrant 8: Roadway Network | $\square$ |
| :--- | ---: |
| 8 A. Weekday Volume (Peak hour total --and-- projected warrants 1, 2 or 3) --or-- | $\square$ |
| 8 B. Weekend Volume (Five hours total) | $\square$ |
| Warrant 9: Grade Crossing | $\square$ |
| 9 A. Grade Crossing within 140 ft --and-- | $\square$ |
| 9 B. Peak-Hour Vehicular Volumes | $\square$ |
| Copyright © 2017 University of Florida, All Rights Reserved $\quad$ HCS7 $^{\text {TM }}$ Warrants Version 7.2.1 Generated: 899/2023 11:36 AM |  |



| Warrant 8: Roadway Network | $\square$ |
| :--- | ---: |
| 8 A. Weekday Volume (Peak hour total --and-- projected warrants 1, 2 or 3) --or-- | $\square$ |
| 8 B. Weekend Volume (Five hours total) | $\square$ |
| Warrant 9: Grade Crossing | $\square$ |
| 9 A. Grade Crossing within 140 ft --and-- | $\square$ |
| 9 B. Peak-Hour Vehicular Volumes | $\square$ |

## Appendix D: LOS Worksheets

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.1 | 0.1 | 0.1 | 0.2 | 0.2 | 0.2 | 0.1 |
| Total Del/Veh (s) | 0.7 | 0.1 | 5.6 | 5.0 | 75.7 | 61.3 | 54.9 | 21.8 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.3 | 0.2 | 0.1 |
| Total Del/Veh (s) | 22.4 | 2.9 | 4.8 | 3.0 | 39.5 | 14.5 | 5.9 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | SBR

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.5 |
| Total Del/Veh (s) | 19.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.6 | 0.4 |
| Total Del/Veh (s) | 1.5 | 34.5 | 21.6 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 48.3 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | WB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 180 | 411 |
| Average Queue (ft) | 56 | 190 |
| 95th Queue (ft) | 132 | 383 |
| Link Distance (ft) | 475 | 1700 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 150 | 21 | 216 |
| Average Queue (ft) | 16 | 1 | 90 |
| 95th Queue (ft) | 82 | 20 | 182 |
| Link Distance (ft) | 475 | 266 | 2774 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 284 | 75 | 108 | 100 | 287 | 109 | 305 | 120 |
| Average Queue (ft) | 61 | 196 | 47 | 75 | 69 | 96 | 46 | 124 | 83 |
| 95th Queue (ft) | 139 | 311 | 80 | 94 | 116 | 216 | 103 | 261 | 140 |
| Link Distance (ft) |  | 266 | 67 | 67 |  | 2703 |  | 1578 |  |
| Upstream Blk Time (\%) |  | 5 | 7 | 41 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 24 | 25 | 148 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 1 | 22 |  |  | 13 | 11 | 2 | 13 | 6 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB |
| :--- | ---: | ---: | ---: |
| Directions Served | T | T | T |
| Maximum Queue (ft) | 2 | 225 | 680 |
| Average Queue (ft) | 0 | 54 | 313 |
| 95th Queue (ft) | 2 | 204 | 634 |
| Link Distance (ft) | 67 |  | 4265 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  | 200 |  |
| Storage Blk Time (\%) |  | 0 | 20 |
| Queuing Penalty (veh) |  | 0 | 72 |

## Network Summary

Network wide Queuing Penalty: 363

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.0 | 0.0 | 0.2 | 0.2 | 0.2 | 0.1 |
| Total Del/Veh (s) | 0.7 | 0.2 | 6.0 | 5.4 | 42.9 | 43.5 | 30.8 | 13.0 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.0 | 0.1 | 0.1 | 0.3 | 0.3 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 25.9 | 11.7 | 5.1 | 3.1 | 72.7 | 76.0 | 24.8 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.1 | 0.3 | 0.0 | 0.0 | 0.5 | 1.1 | 0.4 | 0.4 | 2.6 | 0.6 |
| Total Del/Veh (s) | 46.7 | 26.2 | 20.1 | 26.6 | 13.5 | 8.4 | 152.6 | 155.1 | 141.0 | 36.7 | 30.7 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.5 |
| Total Del/Veh (s) | 52.5 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.5 | 0.2 |
| Total Del/Veh (s) | 1.5 | 47.9 | 23.4 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 0.7 |
| Total Del/Veh (s) | 86.4 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 3 | 164 | 308 |
| Average Queue (ft) | 0 | 54 | 123 |
| 95th Queue (ft) | 3 | 121 | 263 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 317 | 25 | 678 |
| Average Queue (ft) | 88 | 1 | 304 |
| 95th Queue (ft) | 225 | 11 | 691 |
| Link Distance (ft) | 475 | 266 | 2774 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 286 | 67 | 125 | 100 | 1135 | 109 | 200 | 119 |
| Average Queue (ft) | 100 | 255 | 30 | 78 | 66 | 699 | 50 | 69 | 66 |
| 95th Queue (ft) | 178 | 333 | 64 | 103 | 125 | 1431 | 98 | 155 | 122 |
| Link Distance (ft) |  | 266 | 67 | 67 |  | 2703 |  | 1578 |  |
| Upstream BIk Time (\%) |  | 15 | 2 | 50 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 108 | 6 | 159 |  |  |  |  | 95 |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 9 |
| Storage Blk Time (\%) | 4 | 32 |  |  | 6 | 66 | 3 | 5 | 4 |
| Queuing Penalty (veh) | 24 | 42 |  |  | 24 | 63 | 7 | 9 | 7 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB |
| :--- | ---: | ---: | ---: |
| Directions Served | T | T | T |
| Maximum Queue (ft) | 11 | 224 | 831 |
| Average Queue (ft) | 0 | 36 | 365 |
| 95th Queue (ft) | 8 | 171 | 731 |
| Link Distance (ft) | 67 |  | 4265 |
| Upstream Blk Time (\%) | 0 |  |  |
| Queuing Penalty (veh) | 0 |  |  |
| Storage Bay Dist (ft) |  | 200 |  |
| Storage Blk Time (\%) |  | 0 | 28 |
| Queuing Penalty (veh) |  | 0 | 89 |

Network Summary
Network wide Queuing Penalty: 537

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.1 | 0.1 | 30.0 | 37.4 | 34.5 | 7.1 |
| Total Del/Veh (s) | 0.7 | 0.1 | 6.1 | 5.7 | 475.1 | 473.4 | 415.4 | 111.1 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.0 | 0.0 | 0.0 | 0.3 | 0.2 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 28.4 | 5.5 | 4.9 | 3.0 | 38.2 | 21.8 | 7.7 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.8 | 0.3 | 0.4 | 0.0 | 0.0 | 0.0 | 1.0 | 0.3 | 0.3 | 2.6 | 0.9 |
| Total Del/Veh (s) | 48.2 | 31.0 | 23.6 | 27.6 | 13.0 | 8.5 | 41.9 | 33.2 | 18.1 | 51.7 | 55.4 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | :---: |
| Denied Del/Veh (s) | 0.7 |
| Total DelVeh (s) | 30.2 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 1.2 | 0.8 |
| Total Del/Veh (s) | 1.6 | 192.4 | 123.8 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 4.6 |
| Total Del/Veh (s) | 155.9 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 7 | 206 | 1318 |
| Average Queue (ft) | 0 | 67 | 905 |
| 95th Queue (ft) | 5 | 149 | 1821 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  |  | 15 |
| Queuing Penalty (veh) |  |  | 0 |

Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)
Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 184 | 15 | 302 |
| Average Queue (ft) | 35 | 1 | 110 |
| 95th Queue (ft) | 117 | 7 | 232 |
| Link Distance (ft) | 475 | 266 | 2774 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 285 | 83 | 126 | 100 | 284 | 109 | 684 | 120 |
| Average Queue (ft) | 89 | 234 | 60 | 84 | 77 | 114 | 50 | 341 | 106 |
| 95th Queue (ft) | 170 | 328 | 88 | 112 | 118 | 247 | 118 | 632 | 147 |
| Link Distance (ft) |  | 266 | 67 | 67 |  | 2703 |  | 1578 |  |
| Upstream BIk Time (\%) |  | 10 | 21 | 55 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 59 | 91 | 236 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 1 | 32 |  |  | 19 | 14 | 4 | 35 | 18 |
| Queuing Penalty (veh) | 7 | 32 |  |  | 25 | 15 | 19 | 106 | 52 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB |
| :--- | ---: | ---: | ---: |
| Directions Served | T | T | T |
| Maximum Queue (ft) | 21 | 225 | 2859 |
| Average Queue (ft) | 1 | 132 | 1606 |
| 95th Queue (ft) | 11 | 301 | 3290 |
| Link Distance (ft) | 67 |  | 4265 |
| Upstream Blk Time (\%) | 0 |  | 2 |
| Queuing Penalty (veh) | 0 |  | 0 |
| Storage Bay Dist (ft) |  | 200 |  |
| Storage Blk Time (\%) |  | 0 | 53 |
| Queuing Penalty (veh) |  | 1 | 227 |

Network Summary
Network wide Queuing Penalty: 871

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.3 | 0.2 | 0.0 | 0.0 | 0.2 | 0.1 | 0.2 | 0.2 |
| Total Del/Veh (s) | 1.3 | 0.4 | 8.5 | 7.9 | 111.0 | 103.2 | 97.7 | 25.4 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.4 | 0.3 | 0.1 | 0.1 | 15.8 | 29.7 | 6.8 |
| Total Del/Veh $(\mathrm{s})$ | 40.2 | 27.9 | 5.2 | 3.3 | 579.1 | 597.8 | 150.1 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 180.7 | 198.7 | 190.9 | 2.7 | 0.6 |
| Total Del/Veh (s) | 59.8 | 29.5 | 23.8 | 26.8 | 15.5 | 9.2 | 544.6 | 547.6 | 540.9 | 43.8 | 33.3 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 51.2 |
| Total Del/Veh (s) | 155.5 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.6 | 0.3 |
| Total Del/Veh (s) | 1.4 | 138.5 | 67.9 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 51.0 |
| Total Del/Veh (s) | 282.5 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 55 | 226 | 499 |
| Average Queue (ft) | 4 | 85 | 221 |
| 95th Queue (ft) | 31 | 169 | 471 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 468 | 30 | 2526 |
| Average Queue (ft) | 213 | 2 | 1748 |
| 95th Queue (ft) | 453 | 13 | 3107 |
| Link Distance (ft) | 475 | 266 | 2774 |
| Upstream Blk Time (\%) | 1 |  | 18 |
| Queuing Penalty (veh) | 6 |  | 0 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 288 | 72 | 125 | 100 | 2759 | 109 | 251 | 120 |
| Average Queue (ft) | 132 | 274 | 33 | 83 | 62 | 2415 | 55 | 86 | 74 |
| 95th Queue (ft) | 176 | 307 | 67 | 111 | 127 | 3285 | 109 | 195 | 127 |
| Link Distance (ft) |  | 266 | 67 | 67 |  | 2703 |  | 1578 |  |
| Upstream BIk Time (\%) |  | 24 | 3 | 58 |  | 62 |  |  |  |
| Queuing Penalty (veh) |  | 218 | 10 | 196 |  | 0 |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 18 | 35 |  |  | 6 | 73 | 5 | 6 | 4 |
| Queuing Penalty (veh) | 120 | 75 |  |  | 31 | 70 | 14 | 14 | 7 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB |
| :--- | ---: | ---: | ---: |
| Directions Served | T | T | T |
| Maximum Queue (ft) | 4 | 225 | 1540 |
| Average Queue (ft) | 0 | 59 | 885 |
| 95th Queue (ft) | 4 | 221 | 1676 |
| Link Distance (ft) | 67 |  | 4265 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  | 200 |  |
| Storage Blk Time (\%) |  | 0 | 55 |
| Queuing Penalty (veh) | 0 | 185 |  |

Network Summary
Network wide Queuing Penalty: 945

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.1 | 0.1 | 0.1 | 0.2 | 0.2 | 0.2 | 0.1 |
| Total Del/Veh (s) | 3.4 | 0.1 | 5.9 | 5.4 | 118.5 | 134.6 | 107.3 | 33.2 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 7.8 | 0.7 | 0.1 | 0.1 | 0.3 | 0.2 | 0.2 |
| Total Del/Veh $(\mathrm{s})$ | 31.8 | 19.8 | 5.1 | 3.2 | 119.0 | 77.2 | 20.2 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 1.4 | 0.8 | 1.7 | 0.0 | 0.0 | 0.0 | 0.9 | 0.3 | 0.3 | 2.6 | 0.7 |
| Denied Del/Veh (s) | 37.0 | 32.8 | 27.9 | 23.4 | 10.6 | 4.9 | 39.1 | 38.9 | 28.2 | 39.1 | 38.7 |
| Total Del/Veh $(\mathrm{s})$ |  | 24.2 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 24.9 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.7 | 0.1 | 0.1 | 0.5 |
| Total Del/Veh (s) | 3.3 | 120.8 | 867.1 | 1438.7 | 78.1 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.2 |
| Total Del/Veh (s) | 101.8 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 75 | 178 | 533 |
| Average Queue (ft) | 4 | 60 | 274 |
| 95th Queue (ft) | 42 | 137 | 570 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 459 | 12 | 715 |
| Average Queue (ft) | 70 | 0 | 216 |
| 95th Queue (ft) | 288 | 6 | 625 |
| Link Distance (ft) | 475 | 267 | 2744 |
| Upstream Blk Time (\%) | 2 |  |  |
| Queuing Penalty (veh) | 7 |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 149 | 284 | 77 | 112 | 100 | 399 | 107 | 432 | 120 |
| Average Queue (ft) | 67 | 220 | 48 | 73 | 70 | 122 | 49 | 147 | 86 |
| 95th Queue (ft) | 149 | 334 | 89 | 110 | 120 | 301 | 110 | 343 | 141 |
| Link Distance (ft) |  | 267 | 67 | 67 |  | 2672 |  | 1543 |  |
| Upstream Blk Time (\%) |  | 17 | 11 | 42 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 88 | 40 | 149 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 0 | 33 |  |  | 13 | 18 | 5 | 13 | 9 |
| Queuing Penalty (veh) | 1 | 25 |  |  | 16 | 20 | 15 | 29 | 16 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 70 | 225 | 2179 | 33 | 23 |
| Average Queue (ft) | 10 | 78 | 909 | 9 | 7 |
| 95th Queue (ft) | 48 | 244 | 2184 | 33 | 24 |
| Link Distance (ft) | 67 |  | 4280 | 276 | 226 |
| Upstream Blk Time (\%) | 8 |  |  |  |  |
| Queuing Penalty (veh) | 36 |  |  |  |  |
| Storage Bay Dist (ft) |  | 200 |  |  |  |
| Storage Blk Time (\%) |  | 0 | 38 |  |  |
| Queuing Penalty (veh) |  | 0 | 135 |  |  |

Network Summary
Network wide Queuing Penalty: 577

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.0 | 0.0 | 0.2 | 0.2 | 0.2 | 0.1 |
| Total Del/Veh (s) | 5.6 | 4.2 | 6.3 | 5.6 | 62.7 | 53.2 | 47.5 | 18.4 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 1.0 | 0.2 | 0.1 | 0.1 | 1.2 | 2.1 | 0.7 |
| Total Del/Veh $(\mathrm{s})$ | 47.6 | 34.1 | 5.7 | 3.7 | 284.2 | 261.9 | 80.7 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.2 | 0.4 | 0.2 | 0.0 | 0.1 | 0.0 | 1.0 | 0.5 | 0.5 | 2.7 | 0.5 |
| Denied Del/Veh (s) | 52.8 | 34.7 | 26.5 | 27.6 | 14.0 | 9.8 | 286.6 | 297.7 | 287.6 | 52.7 | 31.4 |
| Total Del/Veh (s) |  | 17.7 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.5 |
| Total Del/Veh (s) | 92.0 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.6 | 0.1 | 0.1 | 0.3 |
| Total Del/Veh (s) | 2.9 | 175.4 | 1010.2 | 1029.1 | 90.7 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.2 |
| Total Del/Veh (s) | 192.7 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 190 | 171 | 388 |
| Average Queue (ft) | 14 | 54 | 150 |
| 95th Queue (ft) | 93 | 122 | 335 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 486 | 48 | 1708 |
| Average Queue (ft) | 161 | 4 | 870 |
| 95th Queue (ft) | 405 | 49 | 1982 |
| Link Distance (ft) | 475 | 267 | 2744 |
| Upstream Blk Time (\%) | 5 | 0 | 3 |
| Queuing Penalty (veh) | 18 | 3 | 0 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 287 | 70 | 126 | 100 | 1910 | 109 | 324 | 120 |
| Average Queue (ft) | 102 | 268 | 30 | 78 | 63 | 1254 | 54 | 83 | 64 |
| 95th Queue (ft) | 181 | 321 | 67 | 121 | 128 | 2308 | 108 | 211 | 121 |
| Link Distance (ft) |  | 267 | 67 | 67 |  | 2672 |  | 1543 |  |
| Upstream Blk Time (\%) |  | 26 | 3 | 50 |  | 0 |  |  |  |
| Queuing Penalty (veh) |  | 188 | 8 | 162 |  | 0 |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 4 | 41 |  |  | 4 | 73 | 7 | 5 | 3 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 78 | 224 | 2013 | 35 | 21 |
| Average Queue (ft) | 10 | 45 | 1065 | 10 | 6 |
| 95th Queue (ft) | 52 | 188 | 2263 | 31 | 24 |
| Link Distance (ft) | 67 |  | 4280 | 276 | 226 |
| Upstream Blk Time (\%) | 8 |  |  |  |  |
| Queuing Penalty (veh) | 59 |  |  |  |  |
| Storage Bay Dist (ft) |  | 200 |  |  |  |
| Storage Blk Time (\%) |  | 0 | 53 |  |  |
| Queuing Penalty (veh) |  | 0 | 167 |  |  |

Network Summary
Network wide Queuing Penalty: 796

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.1 | 0.1 | 8.1 | 6.2 | 8.4 | 2.0 |
| Total Del/Veh (s) | 7.2 | 8.4 | 5.9 | 5.5 | 387.2 | 361.9 | 381.1 | 98.5 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 4.6 | 0.4 | 0.1 | 0.1 | 0.3 | 0.2 | 0.2 |
| Total Del/Veh $(\mathrm{s})$ | 55.8 | 29.7 | 4.7 | 3.0 | 146.3 | 156.4 | 36.3 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | SBR

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 1.8 |
| Total Del/Veh (s) | 45.0 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.4 | 112.5 | 0.1 | 0.1 | 71.4 |
| Total Del/Veh (s) | 3.3 | 469.3 | 769.7 | 1495.5 | 300.9 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 43.3 |
| Total Del/Veh (s) | 272.6 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 194 | 196 | 1311 |
| Average Queue (ft) | 13 | 62 | 788 |
| 95th Queue (ft) | 88 | 143 | 1502 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  |  | 5 |
| Queuing Penalty (veh) |  |  | 0 |

Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)
Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 487 | 8 | 928 |
| Average Queue (ft) | 122 | 0 | 398 |
| 95th Queue (ft) | 371 | 4 | 922 |
| Link Distance (ft) | 475 | 267 | 2744 |
| Upstream Blk Time (\%) | 5 |  |  |
| Queuing Penalty (veh) | 18 |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 285 | 82 | 120 | 100 | 535 | 109 | 1083 | 120 |
| Average Queue (ft) | 90 | 256 | 56 | 77 | 75 | 168 | 56 | 520 | 104 |
| 95th Queue (ft) | 174 | 334 | 94 | 120 | 121 | 419 | 122 | 1120 | 154 |
| Link Distance (ft) |  | 267 | 67 | 67 |  | 2672 |  | 1543 |  |
| Upstream Blk Time (\%) |  | 28 | 22 | 52 |  |  |  | 3 |  |
| Queuing Penalty (veh) |  | 166 | 96 | 221 |  |  |  | 0 |  |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 3 | 46 |  |  | 20 | 21 | 8 | 40 | 19 |
| Queuing Penalty (veh) | 15 | 46 |  |  | 27 | 23 | 35 | 121 | 52 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 72 | 225 | 4339 | 37 | 21 |
| Average Queue (ft) | 10 | 113 | 3454 | 9 | 7 |
| 95th Queue (ft) | 49 | 282 | 5102 | 31 | 28 |
| Link Distance (ft) | 67 |  | 4280 | 276 | 226 |
| Upstream Blk Time (\%) | 8 |  | 39 |  |  |
| Queuing Penalty (veh) | 41 |  | 0 |  |  |
| Storage Bay Dist (ft) |  | 200 |  |  |  |
| Storage Blk Time (\%) |  | 1 | 61 |  |  |
| Queuing Penalty (veh) |  | 3 | 260 |  |  |

Network Summary
Network wide Queuing Penalty: 1123

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 5.4 | 7.4 | 0.0 | 0.0 | 67.2 | 83.2 | 72.8 | 15.8 |
| Total Del/Veh (s) | 30.5 | 32.0 | 11.5 | 10.1 | 673.1 | 597.5 | 641.0 | 151.9 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.4 | 2.9 | 0.1 | 0.1 | 168.0 | 196.9 | 47.7 |
| Total Del/Veh $(\mathrm{s})$ | 85.2 | 64.9 | 5.1 | 3.3 | 987.6 | 922.0 | 228.2 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Tonied Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 349.2 | 365.4 | 349.4 | 2.8 | 0.6 |
| Del/Veh (s) | 65.3 | 38.1 | 30.5 | 31.5 | 15.9 | 10.1 | 649.0 | 640.0 | 632.6 | 60.7 | 37.6 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 99.4 |
| Total Del/Veh (s) | 180.4 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.5 | 0.1 | 0.3 |  |
| Total Del/Veh (s) | 2.8 | 292.0 | 1400.1 |  | 151.6 |

Total Network Performance

|  |  |
| :--- | :--- |
| Denied Del/Veh (s) | 121.8 |
| Total Del/Veh (s) | 436.2 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 604 | 324 | 1601 |
| Average Queue (ft) | 119 | 101 | 1054 |
| 95th Queue (ft) | 457 | 235 | 1930 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) | 6 | 0 | 25 |
| Queuing Penalty (veh) | 0 | 0 | 0 |

Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)
Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR |
| Maximum Queue (ft) | 492 | 32 | 2795 |
| Average Queue (ft) | 345 | 2 | 2313 |
| 95th Queue (ft) | 589 | 19 | 3235 |
| Link Distance (ft) | 475 | 267 | 2744 |
| Upstream Blk Time (\%) | 18 |  | 45 |
| Queuing Penalty (veh) | 99 |  | 0 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 288 | 72 | 128 | 100 | 2733 | 108 | 419 | 120 |
| Average Queue (ft) | 118 | 274 | 33 | 80 | 55 | 2571 | 56 | 108 | 69 |
| 95th Queue (ft) | 193 | 310 | 68 | 124 | 121 | 3048 | 108 | 333 | 129 |
| Link Distance (ft) |  | 267 | 67 | 67 |  | 2672 |  | 1543 |  |
| Upstream Blk Time (\%) |  | 34 | 3 | 55 |  | 76 |  |  |  |
| Queuing Penalty (veh) |  | 308 | 11 | 185 |  | 0 |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 17 | 43 |  |  | 6 | 76 | 9 | 6 | 5 |
| Queuing Penalty (veh) | 118 | 93 |  |  | 33 | 72 | 24 | 13 | 8 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 75 | 225 | 2848 | 22 | 12 |
| Average Queue (ft) | 10 | 67 | 1729 | 7 | 2 |
| 95th Queue (ft) | 52 | 234 | 3273 | 29 | 13 |
| Link Distance (ft) | 67 |  | 4280 | 276 | 226 |
| Upstream Blk Time (\%) | 8 |  |  |  |  |
| Queuing Penalty (veh) | 70 |  |  |  |  |
| Storage Bay Dist (ft) |  | 200 |  |  |  |
| Storage Blk Time (\%) |  | 0 | 65 |  |  |
| Queuing Penalty (veh) |  | 0 | 219 |  |  |

Network Summary
Network wide Queuing Penalty: 1252

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.2 | 0.2 | 0.1 | 0.2 | 0.4 | 0.2 | 0.2 |
| Total Del/Veh (s) | 5.3 | 2.3 | 13.2 | 11.8 | 34.4 | 32.2 | 25.0 | 17.2 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 27.5 | 4.8 | 7.1 | 4.5 | 54.2 | 7.1 | 6.3 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 2.1 | 0.3 | 0.4 | 3.3 | 0.7 |
| Denied Del/Veh (s) | 23.6 | 13.5 | 5.1 | 13.6 | 8.0 | 4.3 | 53.9 | 49.5 | 23.1 | 42.1 | 48.1 |
| Total Del/Veh (s) | 22.4 |  |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.6 |
| Total Del/Veh (s) | 19.5 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.0 | 16.0 | 10.0 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 0.9 |
| Total Del/Veh (s) | 41.9 |

## MOVEMENT SUMMARY

## $\forall$ Site: 101 [Cook Rd/l-5 SB (Site Folder: Roundabout Alternative <br> - 2028 AM Peak Hour)]

New Site<br>Site Category: (None)<br>Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | INPUT VOLUMES |  | DEMAND FLOWS | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | 95\% BACK OF QUEUE |  | Prop. Que | Effective Stop Rate | Aver. Aver. <br> No. Speed Cycles <br> mph |  |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 535 | 16.0 | 569 | 16.0 | 0.608 | 9.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.64 | 0.00 | 32.3 |
| $6 \quad$ T1 | 100 | 16.0 | 106 | 16.0 | 0.608 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.64 | 0.00 | 32.6 |
| Approach | 635 | 16.0 | 676 | 16.0 | 0.608 | 8.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.64 | 0.00 | 32.3 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 215 | 6.0 | 229 | 6.0 | 0.383 | 15.8 | LOS B | 2.2 | 58.7 | 0.74 | 0.92 | 0.78 | 24.7 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.383 | 10.8 | LOS B | 2.2 | 58.7 | 0.74 | 0.92 | 0.78 | 31.7 |
| 14 R2 | 25 | 6.0 | 27 | 6.0 | 0.383 | 10.6 | LOS B | 2.2 | 58.7 | 0.74 | 0.92 | 0.78 | 31.0 |
| Approach | 245 | 6.0 | 261 | 6.0 | 0.383 | 15.2 | LOS B | 2.2 | 58.7 | 0.74 | 0.92 | 0.78 | 25.6 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 90 | 16.0 | 96 | 16.0 | 0.211 | 12.2 | LOS B | 1.1 | 32.0 | 0.76 | 0.85 | 0.76 | 29.9 |
| 12 R 2 | 10 | 16.0 | 11 | 16.0 | 0.211 | 12.1 | LOS B | 1.1 | 32.0 | 0.76 | 0.85 | 0.76 | 32.0 |
| Approach | 100 | 16.0 | 106 | 16.0 | 0.211 | 12.2 | LOS B | 1.1 | 32.0 | 0.76 | 0.85 | 0.76 | 30.2 |
| All Vehicles | 980 | 13.5 | 1043 | 13.5 | 0.608 | 10.7 | LOS B | 2.2 | 58.7 | 0.26 | 0.73 | 0.27 | 30.0 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:I23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative.sip9

## MOVEMENT SUMMARY

## $\nabla$ Site: 102 [Cook Rd/l-5 NB (Site Folder: Roundabout Alternative <br> - 2028 AM Peak Hour)]

New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | INPUT VOLUMES |  | DEMAND FLOWS |  | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | 95\% BACK OF QUEUE |  | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 1 | 15.0 | 1 | 15.0 | 0.290 | 12.0 | LOS B | 1.4 | 40.1 | 0.51 | 0.66 | 0.51 | 27.8 |
| 8 T1 | 5 | 15.0 | 5 | 15.0 | 0.290 | 6.9 | LOS A | 1.4 | 40.1 | 0.51 | 0.66 | 0.51 | 35.6 |
| 18 R2 | 230 | 15.0 | 237 | 15.0 | 0.290 | 6.8 | LOS A | 1.4 | 40.1 | 0.51 | 0.66 | 0.51 | 31.4 |
| Approach | 236 | 15.0 | 243 | 15.0 | 0.290 | 6.8 | LOS A | 1.4 | 40.1 | 0.51 | 0.66 | 0.51 | 31.5 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 635 | 12.0 | 655 | 12.0 | 0.789 | 3.8 | LOS A | 13.0 | 357.2 | 0.28 | 0.39 | 0.28 | 29.7 |
| 16 R 2 | 235 | 12.0 | 242 | 12.0 | 0.789 | 3.5 | LOS A | 13.0 | 357.2 | 0.28 | 0.39 | 0.28 | 33.4 |
| Approach | 870 | 12.0 | 897 | 12.0 | 0.789 | 3.7 | LOS A | 13.0 | 357.2 | 0.28 | 0.39 | 0.28 | 31.3 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 5 | 8.0 | 5 | 8.0 | 0.268 | 9.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.43 | 0.00 | 35.7 |
| 2 T1 | 305 | 8.0 | 314 | 8.0 | 0.268 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.43 | 0.00 | 28.5 |
| Approach | 310 | 8.0 | 320 | 8.0 | 0.268 | 4.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.43 | 0.00 | 28.8 |
| All Vehicles | 1416 | 11.6 | 1460 | 11.6 | 0.789 | 4.4 | LOS A | 13.0 | 357.2 | 0.26 | 0.44 | 0.26 | 30.9 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:I23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative.sip9

## MOVEMENT SUMMARY

$\nabla$ Site: 103 [Cook Rd/Old Hwy 99 N (Site Folder: Roundabout

## Alternative - 2028 AM Peak Hour)]

New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | INPUT VOLUMES |  | DEMAND FLOWS | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. <br> Satn <br> v/c | Aver. Delay sec | Level of Service | 95\% BACK OF QUEUE |  | Prop. Que | Effective Stop Rate | Aver. Aver.  <br> No. Speed  <br> Cycles mph |  |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 110 | 22.0 | 116 | 22.0 | 0.173 | 12.7 | LOS B | 0.7 | 20.9 | 0.55 | 0.81 | 0.55 | 27.8 |
| 8 T1 | 65 | 22.0 | 68 | 22.0 | 0.182 | 7.2 | LOS A | 0.8 | 22.5 | 0.55 | 0.69 | 0.55 | 35.3 |
| 18 R2 | 60 | 22.0 | 63 | 22.0 | 0.182 | 7.2 | LOS A | 0.8 | 22.5 | 0.55 | 0.69 | 0.55 | 34.1 |
| Approach | 235 | 22.0 | 247 | 22.0 | 0.182 | 9.8 | LOS A | 0.8 | 22.5 | 0.55 | 0.75 | 0.55 | 31.9 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 95 | 9.0 | 100 | 9.0 | 0.172 | 12.3 | LOS B | 0.6 | 17.2 | 0.46 | 0.76 | 0.46 | 33.2 |
| 6 T1 | 585 | 9.0 | 616 | 9.0 | 0.652 | 7.4 | LOS A | 5.0 | 135.0 | 0.63 | 0.72 | 0.71 | 27.2 |
| 16 R2 | 35 | 9.0 | 37 | 9.0 | 0.652 | 7.3 | LOS A | 5.0 | 135.0 | 0.63 | 0.72 | 0.71 | 34.1 |
| Approach | 715 | 9.0 | 753 | 9.0 | 0.652 | 8.0 | LOS A | 5.0 | 135.0 | 0.61 | 0.73 | 0.68 | 28.6 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 65 | 21.0 | 68 | 21.0 | 0.215 | 18.2 | LOS B | 0.8 | 24.6 | 0.69 | 0.89 | 0.69 | 30.4 |
| 4 T1 | 115 | 21.0 | 121 | 21.0 | 0.508 | 11.5 | LOS B | 3.1 | 91.4 | 0.78 | 0.95 | 0.97 | 33.3 |
| 14 R2 | 165 | 21.0 | 174 | 21.0 | 0.508 | 11.5 | LOS B | 3.1 | 91.4 | 0.78 | 0.95 | 0.97 | 25.1 |
| Approach | 345 | 21.0 | 363 | 21.0 | 0.508 | 12.8 | LOS B | 3.1 | 91.4 | 0.77 | 0.94 | 0.92 | 29.3 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 75 | 13.0 | 79 | 13.0 | 0.146 | 11.5 | LOS B | 0.6 | 15.3 | 0.48 | 0.77 | 0.48 | 30.1 |
| 2 T1 | 350 | 13.0 | 368 | 13.0 | 0.525 | 5.8 | LOS A | 3.2 | 89.1 | 0.58 | 0.68 | 0.61 | 33.9 |
| 12 R 2 | 115 | 13.0 | 121 | 13.0 | 0.525 | 5.7 | LOS A | 3.2 | 89.1 | 0.58 | 0.68 | 0.61 | 32.1 |
| Approach | 540 | 13.0 | 568 | 13.0 | 0.525 | 6.6 | LOS A | 3.2 | 89.1 | 0.57 | 0.69 | 0.59 | 32.9 |
| All Vehicles | 1835 | 14.1 | 1932 | 14.1 | 0.652 | 8.7 | LOS A | 5.0 | 135.0 | 0.62 | 0.76 | 0.68 | 30.2 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\nabla$ Site: 101 [Cook Rd/l-5 SB - Copy (2) (Site Folder: Roundabout
Alternative - 2028 PM Peak Hour)]

```
New Site
Site Category: (None)
Roundabout
```

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | INPUT VOLUMES |  |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \\ & \hline \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec | Level of Service | 95\% BACK OF QUEUE | K OF JE Dist ] ft | Prop. Que | Effective Stop Rate | Aver. Aver.  <br> No. Speed  <br> Cycles mph |  |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 400 | 8.0 | 417 | 8.0 | 0.449 | 9.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.64 | 0.00 | 32.8 |
| $6 \quad$ T1 | 115 | 8.0 | 120 | 8.0 | 0.449 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.64 | 0.00 | 33.1 |
| Approach | 515 | 8.0 | 536 | 8.0 | 0.449 | 8.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.64 | 0.00 | 32.9 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 185 | 6.0 | 193 | 6.0 | 0.276 | 13.2 | LOS B | 1.4 | 37.0 | 0.61 | 0.81 | 0.61 | 25.8 |
| $4 \quad \mathrm{~T} 1$ | 5 | 6.0 | 5 | 6.0 | 0.276 | 8.1 | LOS A | 1.4 | 37.0 | 0.61 | 0.81 | 0.61 | 32.9 |
| 14 R2 | 20 | 6.0 | 21 | 6.0 | 0.276 | 8.0 | LOS A | 1.4 | 37.0 | 0.61 | 0.81 | 0.61 | 32.1 |
| Approach | 210 | 6.0 | 219 | 6.0 | 0.276 | 12.6 | LOS B | 1.4 | 37.0 | 0.61 | 0.81 | 0.61 | 26.7 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 200 | 4.0 | 208 | 4.0 | 0.295 | 8.7 | LOS A | 1.6 | 42.4 | 0.68 | 0.77 | 0.68 | 32.7 |
| 12 R 2 | 10 | 4.0 | 10 | 4.0 | 0.295 | 8.6 | LOS A | 1.6 | 42.4 | 0.68 | 0.77 | 0.68 | 33.9 |
| Approach | 210 | 4.0 | 219 | 4.0 | 0.295 | 8.7 | LOS A | 1.6 | 42.4 | 0.68 | 0.77 | 0.68 | 32.7 |
| All Vehicles | 935 | 6.7 | 974 | 6.7 | 0.449 | 9.3 | LOS A | 1.6 | 42.4 | 0.29 | 0.71 | 0.29 | 31.2 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## $\nabla$ Site: 102 [Cook Rd/I-5 NB - Copy (2) (Site Folder: Roundabout <br> Alternative - 2028 PM Peak Hour)]

```
New Site
Site Category: (None)
Roundabout
```

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { IT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \end{aligned}$ | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service | 95\% QU [ Veh. veh | K OF UE Dist ] ft | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 5 | 6.0 | 5 | 6.0 | 0.476 | 12.7 | LOS B | 2.9 | 76.9 | 0.62 | 0.74 | 0.64 | 27.6 |
| 8 T1 | 1 | 6.0 | 1 | 6.0 | 0.476 | 7.7 | LOS A | 2.9 | 76.9 | 0.62 | 0.74 | 0.64 | 35.3 |
| 18 R2 | 380 | 6.0 | 418 | 6.0 | 0.476 | 7.5 | LOS A | 2.9 | 76.9 | 0.62 | 0.74 | 0.64 | 30.9 |
| Approach | 386 | 6.0 | 424 | 6.0 | 0.476 | 7.6 | LOS A | 2.9 | 76.9 | 0.62 | 0.74 | 0.64 | 30.8 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 505 | 7.0 | 555 | 7.0 | 0.733 | 4.2 | LOS A | 8.5 | 224.8 | 0.41 | 0.44 | 0.41 | 29.3 |
| 16 R2 | 265 | 7.0 | 291 | 7.0 | 0.733 | 4.0 | LOS A | 8.5 | 224.8 | 0.41 | 0.44 | 0.41 | 33.2 |
| Approach | 770 | 7.0 | 846 | 7.0 | 0.733 | 4.2 | LOS A | 8.5 | 224.8 | 0.41 | 0.44 | 0.41 | 31.3 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 35 | 6.0 | 38 | 6.0 | 0.316 | 9.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.46 | 0.00 | 35.4 |
| 2 T1 | 315 | 6.0 | 346 | 6.0 | 0.316 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.46 | 0.00 | 28.1 |
| Approach | 350 | 6.0 | 385 | 6.0 | 0.316 | 4.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.46 | 0.00 | 29.4 |
| All Vehicles | 1506 | 6.5 | 1655 | 6.5 | 0.733 | 5.2 | LOS A | 8.5 | 224.8 | 0.37 | 0.52 | 0.38 | 30.8 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:I23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative.sip9

## MOVEMENT SUMMARY

## $\forall$ Site: 103 [Cook Rd/Old Hwy 99 N - Copy (2) (Site Folder:

## Roundabout Alternative - 2028 PM Peak Hour)]

New Site
Site Category: (None)
Roundabout


Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements ( $\mathrm{v} / \mathrm{c}$ not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

$\square$ Site: 101 [Cook Rd/l-5 SB - Copy (Site Folder: Roundabout
Alternative - 2045 AM Peak Hour)]

```
New Site
Site Category: (None)
Roundabout
```

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | INPUT VOLUMES |  | DEMAND FLOWS |  | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | 95\% BACK OF QUEUE |  | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 535 | 16.0 | 535 | 16.0 | 0.646 | 9.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 32.8 |
| $6 \quad$ T1 | 255 | 16.0 | 255 | 16.0 | 0.646 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 33.2 |
| Approach | 790 | 16.0 | 790 | 16.0 | 0.646 | 7.9 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 0.00 | 32.9 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 225 | 6.0 | 225 | 6.0 | 0.373 | 16.5 | LOS B | 2.3 | 60.8 | 0.79 | 0.92 | 0.81 | 24.4 |
| $4 \quad \mathrm{~T} 1$ | 5 | 6.0 | 5 | 6.0 | 0.373 | 11.5 | LOS B | 2.3 | 60.8 | 0.79 | 0.92 | 0.81 | 31.5 |
| 14 R2 | 35 | 6.0 | 35 | 6.0 | 0.373 | 11.3 | LOS B | 2.3 | 60.8 | 0.79 | 0.92 | 0.81 | 30.8 |
| Approach | 265 | 6.0 | 265 | 6.0 | 0.373 | 15.7 | LOS B | 2.3 | 60.8 | 0.79 | 0.92 | 0.81 | 25.6 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 120 | 16.0 | 120 | 16.0 | 0.212 | 10.6 | LOS B | 1.2 | 33.9 | 0.75 | 0.81 | 0.75 | 30.9 |
| 12 R2 | 10 | 16.0 | 10 | 16.0 | 0.212 | 10.5 | LOS B | 1.2 | 33.9 | 0.75 | 0.81 | 0.75 | 32.7 |
| Approach | 130 | 16.0 | 130 | 16.0 | 0.212 | 10.6 | LOS B | 1.2 | 33.9 | 0.75 | 0.81 | 0.75 | 31.1 |
| All Vehicles | 1185 | 13.8 | 1185 | 13.8 | 0.646 | 9.9 | LOS A | 2.3 | 60.8 | 0.26 | 0.70 | 0.26 | 30.6 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## S Site: 102 [Cook Rd/I-5 NB - Copy (Site Folder: Roundabout

## Alternative - 2045 AM Peak Hour)]

```
New Site
Site Category: (None)
Roundabout
```

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | $\begin{aligned} & \text { IT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \end{aligned}$ | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service | 95\% QU [ Veh. veh | CK OF UE Dist ] ft | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 1 | 15.0 | 1 | 15.0 | 0.269 | 11.8 | LOS B | 1.3 | 37.5 | 0.50 | 0.65 | 0.50 | 28.0 |
| 8 T1 | 5 | 15.0 | 5 | 15.0 | 0.269 | 6.7 | LOS A | 1.3 | 37.5 | 0.50 | 0.65 | 0.50 | 35.7 |
| 18 R2 | 245 | 15.0 | 245 | 15.0 | 0.269 | 6.6 | LOS A | 1.3 | 37.5 | 0.50 | 0.65 | 0.50 | 31.6 |
| Approach | 251 | 15.0 | 251 | 15.0 | 0.269 | 6.6 | LOS A | 1.3 | 37.5 | 0.50 | 0.65 | 0.50 | 31.7 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 790 | 12.0 | 790 | 12.0 | 0.826 | 3.9 | LOS A | 15.2 | 417.0 | 0.35 | 0.38 | 0.35 | 29.1 |
| 16 R2 | 240 | 12.0 | 240 | 12.0 | 0.826 | 3.7 | LOS A | 15.2 | 417.0 | 0.35 | 0.38 | 0.35 | 33.1 |
| Approach | 1030 | 12.0 | 1030 | 12.0 | 0.826 | 3.8 | LOS A | 15.2 | 417.0 | 0.35 | 0.38 | 0.35 | 30.7 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 10 | 8.0 | 10 | 8.0 | 0.266 | 9.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.43 | 0.00 | 35.6 |
| 2 T1 | 340 | 8.0 | 340 | 8.0 | 0.266 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.43 | 0.00 | 28.5 |
| Approach | 350 | 8.0 | 350 | 8.0 | 0.266 | 4.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.43 | 0.00 | 28.9 |
| All Vehicles | 1631 | 11.6 | 1631 | 11.6 | 0.826 | 4.4 | LOS A | 15.2 | 417.0 | 0.30 | 0.44 | 0.30 | 30.6 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## $\nabla$ Site: 103 [Cook Rd/Old Hwy 99 N - Copy (Site Folder:

## Roundabout Alternative - 2045 AM Peak Hour)]

New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | INPUT VOLUMES |  | DEMAND FLOWS | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec | Level of Service | 95\% BACK OF QUEUE |  | Prop. Que | Effective Stop Rate | Aver. Aver.  <br> No. Speed  <br> Cycles mph |  |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 110 | 22.0 | 110 | 22.0 | 0.158 | 12.6 | LOS B | 0.7 | 19.4 | 0.56 | 0.80 | 0.56 | 27.8 |
| 8 T1 | 70 | 22.0 | 70 | 22.0 | 0.166 | 6.8 | LOS A | 0.7 | 21.3 | 0.55 | 0.67 | 0.55 | 35.4 |
| 18 R2 | 65 | 22.0 | 65 | 22.0 | 0.166 | 6.9 | LOS A | 0.7 | 21.3 | 0.55 | 0.67 | 0.55 | 34.1 |
| Approach | 245 | 22.0 | 245 | 22.0 | 0.166 | 9.4 | LOS A | 0.7 | 21.3 | 0.56 | 0.73 | 0.56 | 32.1 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 140 | 9.0 | 140 | 9.0 | 0.202 | 11.8 | LOS B | 0.8 | 21.1 | 0.45 | 0.75 | 0.45 | 33.4 |
| 6 T1 | 670 | 9.0 | 670 | 9.0 | 0.638 | 6.9 | LOS A | 4.8 | 128.2 | 0.61 | 0.67 | 0.67 | 27.3 |
| 16 R2 | 45 | 9.0 | 45 | 9.0 | 0.638 | 6.8 | LOS A | 4.8 | 128.2 | 0.61 | 0.67 | 0.67 | 34.2 |
| Approach | 855 | 9.0 | 855 | 9.0 | 0.638 | 7.7 | LOS A | 4.8 | 128.2 | 0.58 | 0.68 | 0.64 | 29.0 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 65 | 21.0 | 65 | 21.0 | 0.165 | 16.4 | LOS B | 0.7 | 19.8 | 0.69 | 0.89 | 0.69 | 31.1 |
| 4 T1 | 215 | 21.0 | 215 | 21.0 | 0.703 | 14.4 | LOS B | 6.0 | 174.9 | 0.89 | 1.10 | 1.32 | 31.9 |
| 14 R2 | 240 | 21.0 | 240 | 21.0 | 0.703 | 14.4 | LOS B | 6.0 | 174.9 | 0.89 | 1.10 | 1.32 | 23.8 |
| Approach | 520 | 21.0 | 520 | 21.0 | 0.703 | 14.7 | LOS B | 6.0 | 174.9 | 0.86 | 1.07 | 1.24 | 28.6 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 100 | 13.0 | 100 | 13.0 | 0.171 | 11.7 | LOS B | 0.7 | 19.0 | 0.54 | 0.81 | 0.54 | 30.0 |
| 2 T1 | 380 | 13.0 | 380 | 13.0 | 0.516 | 6.3 | LOS A | 3.3 | 91.4 | 0.65 | 0.74 | 0.71 | 33.5 |
| 12 R 2 | 115 | 13.0 | 115 | 13.0 | 0.516 | 6.1 | LOS A | 3.3 | 91.4 | 0.65 | 0.74 | 0.71 | 31.8 |
| Approach | 595 | 13.0 | 595 | 13.0 | 0.516 | 7.2 | LOS A | 3.3 | 91.4 | 0.63 | 0.75 | 0.68 | 32.5 |
| All Vehicles | 2215 | 14.3 | 2215 | 14.3 | 0.703 | 9.4 | LOS A | 6.0 | 174.9 | 0.66 | 0.80 | 0.78 | 30.0 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## $\forall$ Site: 101 [Cook Rd/I-5 SB (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | INPUT VOLUMES |  | DEMAND FLOWS | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | 95\% BACK OF QUEUE |  | Prop. Que | Effective Stop Rate | Aver. Aver. <br> No. Speed Cycles <br> mph |  |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 405 | 8.0 | 405 | 8.0 | 0.415 | 9.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 33.0 |
| 6 T1 | 140 | 8.0 | 140 | 8.0 | 0.415 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 33.2 |
| Approach | 545 | 8.0 | 545 | 8.0 | 0.415 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.63 | 0.00 | 33.0 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 190 | 6.0 | 190 | 6.0 | 0.243 | 12.6 | LOS B | 1.3 | 33.2 | 0.59 | 0.78 | 0.59 | 26.0 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.243 | 7.6 | LOS A | 1.3 | 33.2 | 0.59 | 0.78 | 0.59 | 33.2 |
| 14 R2 | 25 | 6.0 | 25 | 6.0 | 0.243 | 7.4 | LOS A | 1.3 | 33.2 | 0.59 | 0.78 | 0.59 | 32.4 |
| Approach | 220 | 6.0 | 220 | 6.0 | 0.243 | 11.9 | LOS B | 1.3 | 33.2 | 0.59 | 0.78 | 0.59 | 27.1 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 370 | 4.0 | 370 | 4.0 | 0.437 | 8.7 | LOS A | 2.9 | 75.3 | 0.73 | 0.81 | 0.77 | 32.6 |
| 12 R 2 | 10 | 4.0 | 10 | 4.0 | 0.437 | 8.6 | LOS A | 2.9 | 75.3 | 0.73 | 0.81 | 0.77 | 33.9 |
| Approach | 380 | 4.0 | 380 | 4.0 | 0.437 | 8.7 | LOS A | 2.9 | 75.3 | 0.73 | 0.81 | 0.77 | 32.7 |
| All Vehicles | 1145 | 6.3 | 1145 | 6.3 | 0.437 | 9.1 | LOS A | 2.9 | 75.3 | 0.36 | 0.72 | 0.37 | 31.5 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## $\square$ Site: 102 [Cook Rd/I-5 NB (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  |  | $\begin{aligned} & \text { JT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ | DEMAND FLOWS |  | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | 95\% BACK OF QUEUE |  | Prop. Que | Effective Stop Rate | Aver. Aver. <br> No. Speed Cycles <br> mph |  |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 5 | 6.0 | 5 | 6.0 | 0.419 | 13.0 | LOS B | 2.5 | 64.4 | 0.65 | 0.76 | 0.65 | 27.5 |
| 8 | T1 | 1 | 6.0 | 1 | 6.0 | 0.419 | 7.9 | LOS A | 2.5 | 64.4 | 0.65 | 0.76 | 0.65 | 35.2 |
| 18 | R2 | 380 | 6.0 | 380 | 6.0 | 0.419 | 7.8 | LOS A | 2.5 | 64.4 | 0.65 | 0.76 | 0.65 | 30.7 |
| Appr | ach | 386 | 6.0 | 386 | 6.0 | 0.419 | 7.9 | LOS A | 2.5 | 64.4 | 0.65 | 0.76 | 0.65 | 30.6 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 | T1 | 535 | 7.0 | 535 | 7.0 | 0.634 | 4.0 | LOS A | 5.7 | 149.3 | 0.32 | 0.44 | 0.32 | 29.9 |
| 16 | R2 | 270 | 7.0 | 270 | 7.0 | 0.634 | 3.8 | LOS A | 5.7 | 149.3 | 0.32 | 0.44 | 0.32 | 33.5 |
| Appr | ach | 805 | 7.0 | 805 | 7.0 | 0.634 | 3.9 | LOS A | 5.7 | 149.3 | 0.32 | 0.44 | 0.32 | 31.8 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 45 | 6.0 | 45 | 6.0 | 0.396 | 9.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 35.5 |
| 2 | T1 | 485 | 6.0 | 485 | 6.0 | 0.396 | 4.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 28.2 |
| Appr | oach | 530 | 6.0 | 530 | 6.0 | 0.396 | 4.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.45 | 0.00 | 29.3 |
| All V | hicles | 1721 | 6.5 | 1721 | 6.5 | 0.634 | 5.1 | LOS A | 5.7 | 149.3 | 0.30 | 0.52 | 0.30 | 30.8 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\checkmark$ Site: 103 [Cook Rd/Old Hwy 99 N (Site Folder: Roundabout

## Alternative - 2045 PM Peak Hour)]

New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{aligned} & \text { IN } \\ & \text { VOL } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | $\begin{aligned} & \text { JT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { DEN } \\ & \text { FLC } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | $\begin{gathered} \hline \text { ND } \\ \text { NS } \\ \text { HV ] } \\ \% \end{gathered}$ | Deg. Satn v/c | Aver. <br> Delay <br> sec | Level of Service | $\begin{gathered} 95 \% \\ \text { Q } \\ \text { Q Veh. } \\ \text { veh } \end{gathered}$ | $\begin{aligned} & \text { CK OF } \\ & \text { UE } \\ & \text { Dist ] } \\ & \mathrm{ft} \end{aligned}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 95 | 6.0 | 95 | 6.0 | 0.179 | 14.1 | LOS B | 0.7 | 19.0 | 0.64 | 0.88 | 0.64 | 27.1 |
| 8 | T1 | 320 | 6.0 | 320 | 6.0 | 0.617 | 9.8 | LOS A | 4.6 | 121.3 | 0.79 | 0.97 | 1.03 | 34.5 |
| 18 | R2 | 215 | 6.0 | 215 | 6.0 | 0.617 | 9.8 | LOS A | 4.6 | 121.3 | 0.79 | 0.97 | 1.03 | 33.4 |
| Appr | oach | 630 | 6.0 | 630 | 6.0 | 0.617 | 10.4 | LOS B | 4.6 | 121.3 | 0.77 | 0.96 | 0.97 | 33.2 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 55 | 4.0 | 60 | 4.0 | 0.100 | 12.9 | LOS B | 0.4 | 10.3 | 0.57 | 0.81 | 0.57 | 33.1 |
| 6 | T1 | 550 | 4.0 | 598 | 4.0 | 0.679 | 9.3 | LOS A | 6.0 | 154.8 | 0.80 | 0.96 | 1.02 | 26.6 |
| 16 | R2 | 65 | 4.0 | 65 | 4.0 | 0.679 | 9.3 | LOS A | 6.0 | 154.8 | 0.80 | 0.96 | 1.02 | 33.7 |
| Appr | ach | 670 | 4.0 | 723 | 4.0 | 0.679 | 9.6 | LOS A | 6.0 | 154.8 | 0.78 | 0.95 | 0.98 | 28.1 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 70 | 10.0 | 70 | 10.0 | 0.137 | 14.1 | LOS B | 0.6 | 16.1 | 0.65 | 0.87 | 0.65 | 32.4 |
| 4 | T1 | 95 | 10.0 | 95 | 10.0 | 0.316 | 7.6 | LOS A | 1.8 | 47.4 | 0.70 | 0.77 | 0.70 | 35.3 |
| 14 | R2 | 160 | 10.0 | 160 | 10.0 | 0.316 | 7.6 | LOS A | 1.8 | 47.4 | 0.70 | 0.77 | 0.70 | 26.9 |
| Appr | ach | 325 | 10.0 | 325 | 10.0 | 0.316 | 9.0 | LOS A | 1.8 | 47.4 | 0.69 | 0.79 | 0.69 | 31.0 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 215 | 6.0 | 215 | 6.0 | 0.258 | 10.0 | LOS B | 1.1 | 29.8 | 0.42 | 0.71 | 0.42 | 31.3 |
| 2 | T1 | 580 | 6.0 | 580 | 6.0 | 0.567 | 4.8 | LOS A | 3.7 | 97.7 | 0.52 | 0.55 | 0.52 | 34.3 |
| 12 | R2 | 100 | 6.0 | 100 | 6.0 | 0.567 | 4.6 | LOS A | 3.7 | 97.7 | 0.52 | 0.55 | 0.52 | 32.6 |
| Approach |  | 895 | 6.0 | 895 | 6.0 | 0.567 | 6.0 | LOS A | 3.7 | 97.7 | 0.50 | 0.59 | 0.50 | 33.3 |
| All Vehicles |  | 2520 | 6.0 | 2573 | 5.9 | 0.679 | 8.5 | LOS A | 6.0 | 154.8 | 0.67 | 0.80 | 0.77 | 31.5 |

Site Level of Service (LOS) Method: Delay \& Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and $\mathrm{v} / \mathrm{c}$ ratio (degree of saturation) per movement.
Intersection and Approach LOS values are based on average delay for all movements (v/c not used).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: HCM Queue Formula.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

## $\nabla$ Site: 101 [Cook Rd/l-5 SB (Site Folder: Roundabout Alternative - 2028 AM Peak Hour)]

-a Network: N101 [2028 AM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | DEM <br> [ Total veh/h | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | $\begin{aligned} & \text { IVAL } \\ & \text { WS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | ACK OF <br> E <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 569 | 16.0 | 569 | 16.0 | 0.608 | 11.2 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.3 |
| 6 T1 | 106 | 16.0 | 106 | 16.0 | 0.608 | 11.2 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.6 |
| Approach | 676 | 16.0 | 676 | 16.0 | 0.608 | 11.2 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.3 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 229 | 6.0 | 229 | 6.0 | 0.464 | 13.0 | LOS B | 1.0 | 27.0 | 0.74 | 0.81 | 0.93 | 23.2 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.464 | 13.0 | LOS B | 1.0 | 27.0 | 0.74 | 0.81 | 0.93 | 29.1 |
| 14 R2 | 27 | 6.0 | 27 | 6.0 | 0.464 | 13.0 | LOS B | 1.0 | 27.0 | 0.74 | 0.81 | 0.93 | 28.4 |
| Approach | 261 | 6.0 | 261 | 6.0 | 0.464 | 13.0 | LOS B | 1.0 | 27.0 | 0.74 | 0.81 | 0.93 | 24.2 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 96 | 16.0 | 96 | 16.0 | 0.259 | 11.5 | LOS B | 0.5 | 12.9 | 0.76 | 0.76 | 0.76 | 25.9 |
| 12 R2 | 11 | 16.0 | 11 | 16.0 | 0.259 | 11.5 | LOS B | 0.5 | 12.9 | 0.76 | 0.76 | 0.76 | 30.4 |
| Approach | 106 | 16.0 | 106 | 16.0 | 0.259 | 11.5 | LOS B | 0.5 | 12.9 | 0.76 | 0.76 | 0.76 | 26.6 |
| All Vehicles | 1043 | 13.5 | 1043 | 13.5 | 0.608 | 11.7 | LOS B | 1.0 | 27.0 | 0.26 | 0.28 | 0.31 | 29.5 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## $\forall$ Site: 102 [Cook Rd/l-5 NB (Site Folder: Roundabout Alternative - 2028 AM Peak Hour)]

마 Network: N101 [2028 AM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { VD } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | VAL WS HV ] \% | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service | AVERA Q <br> [ Veh. veh | BACK OF JE Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. <br> Speed <br> mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L 2 | 1 | 15.0 | 1 | 15.0 | 0.572 | 17.9 | LOS B | 5.0 | 140.8 | 0.51 | 0.55 | 0.74 | 22.4 |
| 8 T1 | 5 | 15.0 | 5 | 15.0 | 0.572 | 17.9 | LOS B | 5.0 | 140.8 | 0.51 | 0.55 | 0.74 | 28.5 |
| 18 R2 | 237 | 15.0 | 237 | 15.0 | 0.572 | 17.9 | LOS B | 5.0 | 140.8 | 0.51 | 0.55 | 0.74 | 22.4 |
| Approach | 243 | 15.0 | 243 | 15.0 | 0.572 | 17.9 | LOS B | 5.0 | 140.8 | 0.51 | 0.55 | 0.74 | 22.6 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $6 \quad$ T1 | 655 | 12.0 | 655 | 12.0 | 0.790 | 17.8 | LOS B | 5.2 | 143.6 | 0.28 | 0.08 | 0.28 | 8.2 |
| 16 R2 | 242 | 12.0 | 242 | 12.0 | 0.790 | 17.8 | LOS B | 5.2 | 143.6 | 0.28 | 0.08 | 0.28 | 23.5 |
| Approach | 897 | 12.0 | 897 | 12.0 | 0.790 | 17.8 | LOS B | 5.2 | 143.6 | 0.28 | 0.08 | 0.28 | 14.9 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 5 | 8.0 | 5 | 8.0 | 0.530 | 12.2 | LOS B | 6.7 | 177.5 | 0.00 | 0.00 | 0.00 | 35.7 |
| $2 \quad \mathrm{~T} 1$ | 314 | 8.0 | 314 | 8.0 | 0.530 | 12.2 | LOS B | 6.7 | 177.5 | 0.00 | 0.00 | 0.00 | 28.1 |
| Approach | 320 | 8.0 | 320 | 8.0 | 0.530 | 12.2 | LOS B | 6.7 | 177.5 | 0.00 | 0.00 | 0.00 | 28.5 |
| All Vehicles | 1460 | 11.6 | 1460 | 11.6 | 0.790 | 16.6 | LOS B | 6.7 | 177.5 | 0.26 | 0.14 | 0.29 | 18.3 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

$\checkmark$ Site: 103 [Cook Rd/Old Hwy 99 N (Site Folder: Roundabout Alternative - 2028 AM Peak Hour)]

무 Network: N101 [2028 AM
Network (Network Folder:
General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov } \\ & \text { ID } \end{aligned}$ |  |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \\ & \hline \end{aligned}$ | ARR FLO [ Tota veh/h | IVAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAC <br> [ Veh. veh | ACK OF <br> JE <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 116 | 22.0 | 116 | 22.0 | 0.213 | 8.1 | LOS A | 0.3 | 7.7 | 0.53 | 0.51 | 0.53 | 25.4 |
| 8 | T1 | 68 | 22.0 | 68 | 22.0 | 0.262 | 8.7 | LOS A | 6.7 | 196.3 | 0.52 | 0.50 | 0.52 | 32.5 |
| 18 | R2 | 63 | 22.0 | 63 | 22.0 | 0.262 | 8.7 | LOS A | 6.7 | 196.3 | 0.52 | 0.50 | 0.52 | 27.6 |
| Appr | ach | 247 | 22.0 | 247 | 22.0 | 0.262 | 8.4 | LOS A | 6.7 | 196.3 | 0.52 | 0.50 | 0.52 | 28.4 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 100 | 9.0 | 100 | 9.0 | 0.171 | 8.3 | LOS A | 0.2 | 6.3 | 0.41 | 0.36 | 0.41 | 27.5 |
| 6 | T1 | 616 | 9.0 | 616 | 9.0 | 0.814 | 24.1 | LOS C | 1.9 | 50.0 | 0.56 | 0.86 | 1.36 | 4.1 |
| 16 | R2 | 37 | 9.0 | 37 | 9.0 | 0.814 | 24.1 | LOS C | 1.9 | 50.0 | 0.56 | 0.86 | 1.36 | 20.9 |
| Appr | ach | 753 | 9.0 | 753 | 9.0 | 0.814 | 22.0 | LOS C | 1.9 | 50.0 | 0.54 | 0.79 | 1.23 | 9.9 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 68 | 21.0 | 68 | 21.0 | 0.405 | 26.4 | LOS C | 7.2 | 208.9 | 0.69 | 0.77 | 0.92 | 18.4 |
| 4 | T1 | 121 | 21.0 | 121 | 21.0 | 0.566 | 17.4 | LOS B | 1.5 | 44.4 | 0.79 | 0.94 | 1.20 | 28.8 |
| 14 | R2 | 174 | 21.0 | 174 | 21.0 | 0.566 | 17.4 | LOS B | 1.5 | 44.4 | 0.79 | 0.94 | 1.20 | 22.8 |
| Appr | ach | 363 | 21.0 | 363 | 21.0 | 0.566 | 19.1 | LOS B | 7.2 | 208.9 | 0.77 | 0.91 | 1.15 | 24.4 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 79 | 13.0 | 79 | 13.0 | 0.511 | 13.5 | LOS B | 8.2 | 225.0 | 0.48 | 0.46 | 0.58 | 25.9 |
| 2 | T1 | 368 | 13.0 | 368 | 13.0 | 0.511 | 13.1 | LOS B | 8.2 | 225.0 | 0.48 | 0.45 | 0.57 | 10.4 |
| 12 | R2 | 121 | 13.0 | 121 | 13.0 | 0.511 | 12.6 | LOS B | 8.2 | 225.0 | 0.48 | 0.45 | 0.57 | 26.0 |
| Approach |  | 568 | 13.0 | 568 | 13.0 | 0.511 | 13.1 | LOS B | 8.2 | 225.0 | 0.48 | 0.45 | 0.57 | 18.8 |
| All V | icles | 1932 | 14.1 | 1932 | 14.1 | 0.814 | 17.1 | LOS B | 8.2 | 225.0 | 0.56 | 0.68 | 0.93 | 19.2 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing (Site Folder: Roundabout Alternative 2028 AM Peak Hour)]

무 Network: N101 [2028 AM
Network (Network Folder:
General)]

## New Site

Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=2900$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | DEM <br> [ Total veh/h | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | $\begin{aligned} & \text { IVAL } \\ & \text { WS } \\ & \text { I HV ] } \\ & \% \end{aligned}$ | Deg Satn v/c | Aver. Delay sec | Level of Service | AVERAC <br> [ Veh. veh | $\begin{aligned} & \text { BACK OF } \\ & \text { UE } \\ & \text { Dist ] } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.006 | 1169.8 | LOS F | 0.8 | 20.1 | 0.92 | 0.59 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.006 | 1169.8 | LOS F | 0.8 | 20.1 | 0.92 | 0.59 | 0.92 | 1.8 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 753 | 9.0 | 753 | 9.0 | * 0.857 | 60.7 | LOS E | 223.1 | 5979.1 | 0.39 | 0.39 | 0.39 | 14.6 |
| Approach | 753 | 9.0 | 753 | 9.0 | 0.857 | 60.7 | LOS E | 223.1 | 5979.1 | 0.39 | 0.39 | 0.39 | 14.6 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.006 | 1169.8 | LOS F | 0.8 | 20.1 | 0.92 | 0.59 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.006 | 1169.8 | LOS F | 0.8 | 20.1 | 0.92 | 0.59 | 0.92 | 1.8 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 500 | 13.0 | 500 | 13.0 | 0.170 | 20.4 | LOS C | 1.8 | 50.0 | 0.13 | 0.12 | 0.13 | 26.2 |
| Approach | 500 | 13.0 | 500 | 13.0 | 0.170 | 20.4 | LOS C | 1.8 | 50.0 | 0.13 | 0.12 | 0.13 | 26.2 |
| All Vehicles | 1255 | 10.6 | 1255 | 10.6 | 0.857 | 46.5 | LOS D | 223.1 | 5979.1 | 0.29 | 0.28 | 0.29 | 17.4 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: HCM Delay Formula (Geometric Delay is not included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)


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Project: M:I23|1.23051.00-Cook Rd and I-5 Interchange Improvements|Traffic Analysis|Traffic Operations|RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

© Site: 101 [Cook Rd/l-5 SB_no RR (Site Folder: Roundabout Alternative - 2028 AM Peak Hour)]

무 Network: N101 [2028 AM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | VAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> [ Veh. veh | ACK OF E Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 569 | 16.0 | 569 | 16.0 | 0.608 | 11.2 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.3 |
| 6 T1 | 106 | 16.0 | 106 | 16.0 | 0.608 | 11.2 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.6 |
| Approach | 676 | 16.0 | 676 | 16.0 | 0.608 | 11.2 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.3 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $7 \quad$ L2 | 229 | 6.0 | 229 | 6.0 | 0.383 | 10.4 | LOS B | 0.9 | 23.8 | 0.74 | 0.75 | 0.79 | 24.4 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.383 | 10.4 | LOS B | 0.9 | 23.8 | 0.74 | 0.75 | 0.79 | 30.1 |
| 14 R2 | 27 | 6.0 | 27 | 6.0 | 0.383 | 10.4 | LOS B | 0.9 | 23.8 | 0.74 | 0.75 | 0.79 | 29.4 |
| Approach | 261 | 6.0 | 261 | 6.0 | 0.383 | 10.4 | LOS B | 0.9 | 23.8 | 0.74 | 0.75 | 0.79 | 25.3 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 96 | 16.0 | 96 | 16.0 | 0.213 | 10.2 | LOS B | 0.5 | 12.9 | 0.76 | 0.75 | 0.76 | 26.7 |
| 12 R2 | 11 | 16.0 | 11 | 16.0 | 0.213 | 10.2 | LOS B | 0.5 | 12.9 | 0.76 | 0.75 | 0.76 | 30.9 |
| Approach | 106 | 16.0 | 106 | 16.0 | 0.213 | 10.2 | LOS B | 0.5 | 12.9 | 0.76 | 0.75 | 0.76 | 27.3 |
| All Vehicles | 1043 | 13.5 | 1043 | 13.5 | 0.608 | 10.9 | LOS B | 0.9 | 23.8 | 0.26 | 0.26 | 0.27 | 30.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6),
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00 - Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

$\forall \sqrt{ } \sqrt{ }$ Site: 102 [Cook Rd/I-5 NB_no RR (Site Folder: Roundabout Alternative - 2028 AM Peak Hour)]

무 Network: N101 [2028 AM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | VAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | $\begin{aligned} & \text { 3ACK OF } \\ & \text { JE } \\ & \text { Dist ] } \\ & \text { ft } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 1 | 15.0 | 1 | 15.0 | 0.290 | 7.5 | LOS A | 0.6 | 16.1 | 0.51 | 0.42 | 0.51 | 28.3 |
| 8 T1 | 5 | 15.0 | 5 | 15.0 | 0.290 | 7.5 | LOS A | 0.6 | 16.1 | 0.51 | 0.42 | 0.51 | 32.9 |
| 18 R2 | 237 | 15.0 | 237 | 15.0 | 0.290 | 7.5 | LOS A | 0.6 | 16.1 | 0.51 | 0.42 | 0.51 | 28.3 |
| Approach | 243 | 15.0 | 243 | 15.0 | 0.290 | 7.5 | LOS A | 0.6 | 16.1 | 0.51 | 0.42 | 0.51 | 28.4 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 655 | 12.0 | 655 | 12.0 | 0.790 | 17.8 | LOS B | 5.2 | 143.6 | 0.28 | 0.08 | 0.28 | 8.2 |
| 16 R2 | 242 | 12.0 | 242 | 12.0 | 0.790 | 17.8 | LOS B | 5.2 | 143.6 | 0.28 | 0.08 | 0.28 | 23.5 |
| Approach | 897 | 12.0 | 897 | 12.0 | 0.790 | 17.8 | LOS B | 5.2 | 143.6 | 0.28 | 0.08 | 0.28 | 14.9 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 5 | 8.0 | 5 | 8.0 | 0.268 | 5.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 35.7 |
| 2 T1 | 314 | 8.0 | 314 | 8.0 | 0.268 | 5.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 28.1 |
| Approach | 320 | 8.0 | 320 | 8.0 | 0.268 | 5.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 28.5 |
| All Vehicles | 1460 | 11.6 | 1460 | 11.6 | 0.790 | 13.4 | LOS B | 5.2 | 143.6 | 0.26 | 0.12 | 0.26 | 19.3 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6),
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

® Site: 103 [Cook Rd/Old Hwy 99 N _no RR (Site Folder: Roundabout Alternative - 2028 AM Peak Hour)]

마 Network: N101 [2028 AM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  |  | ND WS HV] \% | ARR FLO [ Tota veh/h | VAL WS HV ] \% | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | AVERAG QU [ Veh. veh | ACK OF JE Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 116 | 22.0 | 116 | 22.0 | 0.213 | 8.1 | LOS A | 0.3 | 7.7 | 0.53 | 0.51 | 0.53 | 25.4 |
| 8 | T1 | 68 | 22.0 | 68 | 22.0 | 0.177 | 6.8 | LOS A | 0.3 | 8.3 | 0.52 | 0.48 | 0.52 | 33.4 |
| 18 | R2 | 63 | 22.0 | 63 | 22.0 | 0.177 | 6.8 | LOS A | 0.3 | 8.3 | 0.52 | 0.48 | 0.52 | 28.9 |
| Appr | ach | 247 | 22.0 | 247 | 22.0 | 0.213 | 7.4 | LOS A | 0.3 | 8.3 | 0.53 | 0.50 | 0.53 | 29.0 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 100 | 9.0 | 100 | 9.0 | 0.171 | 8.3 | LOS A | 0.3 | 6.9 | 0.46 | 0.40 | 0.46 | 27.5 |
| 6 | T1 | 616 | 9.0 | 616 | 9.0 | 0.814 | 24.1 | LOS C | 1.9 | 50.0 | 0.63 | 0.96 | 1.52 | 4.1 |
| 16 | R2 | 37 | 9.0 | 37 | 9.0 | 0.814 | 24.1 | LOS C | 1.9 | 50.0 | 0.63 | 0.96 | 1.52 | 20.9 |
| Appr | ach | 753 | 9.0 | 753 | 9.0 | 0.814 | 22.0 | LOS C | 1.9 | 50.0 | 0.61 | 0.89 | 1.38 | 9.9 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 68 | 21.0 | 68 | 21.0 | 0.215 | 15.4 | LOS B | 0.3 | 9.9 | 0.69 | 0.69 | 0.69 | 22.1 |
| 4 | T1 | 121 | 21.0 | 121 | 21.0 | 0.585 | 18.7 | LOS B | 1.5 | 44.5 | 0.78 | 0.95 | 1.26 | 28.4 |
| 14 | R2 | 174 | 21.0 | 174 | 21.0 | 0.585 | 18.7 | LOS B | 1.5 | 44.5 | 0.78 | 0.95 | 1.26 | 22.2 |
| Appr | ach | 363 | 21.0 | 363 | 21.0 | 0.585 | 18.1 | LOS B | 1.5 | 44.5 | 0.77 | 0.90 | 1.15 | 24.8 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 79 | 13.0 | 79 | 13.0 | 0.308 | 7.2 | LOS A | 0.6 | 16.0 | 0.48 | 0.39 | 0.48 | 29.5 |
| 2 | T1 | 368 | 13.0 | 368 | 13.0 | 0.308 | 7.2 | LOS A | 0.6 | 16.0 | 0.48 | 0.39 | 0.48 | 13.9 |
| 12 | R2 | 121 | 13.0 | 121 | 13.0 | 0.308 | 7.1 | LOS A | 0.6 | 16.0 | 0.47 | 0.39 | 0.47 | 29.2 |
| Approach |  | 568 | 13.0 | 568 | 13.0 | 0.308 | 7.2 | LOS A | 0.6 | 16.0 | 0.48 | 0.39 | 0.48 | 22.9 |
| All V | hicles | 1932 | 14.1 | 1932 | 14.1 | 0.814 | 15.0 | LOS B | 1.9 | 50.0 | 0.59 | 0.69 | 0.96 | 20.3 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing_no RR (Site Folder: Roundabout Alternative - 2028 AM Peak Hour)]

미 Network: N101 [2028 AM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=61$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | DEM <br> [ Total veh/h | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | $\begin{aligned} & \text { IVAL } \\ & \text { WS } \\ & \text { I HV ] } \\ & \% \end{aligned}$ | Deg Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> [ Veh. veh | ACK OF <br> E <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.035 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.035 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 753 | 9.0 | 753 | 9.0 | * 0.719 | 2.3 | LOS A | 2.2 | 58.6 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 753 | 9.0 | 753 | 9.0 | 0.719 | 2.3 | LOS A | 2.2 | 58.6 | 0.00 | 0.00 | 0.00 | 40.0 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.035 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.035 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 500 | 13.0 | 500 | 13.0 | 0.152 | 1.8 | LOS A | 0.2 | 6.2 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 500 | 13.0 | 500 | 13.0 | 0.152 | 1.8 | LOS A | 0.2 | 6.2 | 0.00 | 0.00 | 0.00 | 40.0 |
| All Vehicles | 1255 | 10.6 | 1255 | 10.6 | 0.719 | 2.1 | LOS A | 2.2 | 58.6 | 0.00 | 0.00 | 0.00 | 39.9 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

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## MOVEMENT SUMMARY

## $\nabla$ Site: 101 [Cook Rd/l-5 SB (Site Folder: Roundabout Alternative - 2028 PM Peak Hour)]

Network: N102 [2028 PM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | DEM <br> [ Total veh/h | $\begin{aligned} & \text { ND } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | VAL <br> NS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | ACK OF <br> E <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 417 | 8.0 | 417 | 8.0 | 0.449 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.8 |
| 6 T1 | 120 | 8.0 | 120 | 8.0 | 0.449 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.1 |
| Approach | 536 | 8.0 | 536 | 8.0 | 0.449 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.9 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $7 \quad$ L2 | 193 | 6.0 | 193 | 6.0 | 0.519 | 16.0 | LOS B | 0.7 | 19.2 | 0.61 | 0.68 | 0.85 | 21.9 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.519 | 16.0 | LOS B | 0.7 | 19.2 | 0.61 | 0.68 | 0.85 | 28.0 |
| 14 R2 | 21 | 6.0 | 21 | 6.0 | 0.519 | 16.0 | LOS B | 0.7 | 19.2 | 0.61 | 0.68 | 0.85 | 27.4 |
| Approach | 219 | 6.0 | 219 | 6.0 | 0.519 | 16.0 | LOS B | 0.7 | 19.2 | 0.61 | 0.68 | 0.85 | 22.9 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 208 | 4.0 | 208 | 4.0 | 0.579 | 20.1 | LOS C | 0.9 | 23.7 | 0.68 | 0.82 | 1.06 | 21.8 |
| 12 R2 | 10 | 4.0 | 10 | 4.0 | 0.579 | 20.1 | LOS C | 0.9 | 23.7 | 0.68 | 0.82 | 1.06 | 27.4 |
| Approach | 219 | 4.0 | 219 | 4.0 | 0.579 | 20.1 | LOS C | 0.9 | 23.7 | 0.68 | 0.82 | 1.06 | 22.2 |
| All Vehicles | 974 | 6.7 | 974 | 6.7 | 0.579 | 12.3 | LOS B | 0.9 | 23.7 | 0.29 | 0.34 | 0.43 | 27.8 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

## $\nabla$ Site: 102 [Cook Rd/l-5 NB (Site Folder: Roundabout Alternative - 2028 PM Peak Hour)]

미 Network: N102 [2028 PM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{\|l\|} \hline \text { Mov } \\ \hline \text { ID } \\ \hline \end{array}$ |  |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \\ & \hline \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | VAL NS HV ] \% | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service | AVERAC <br> [ Veh. veh | ACK OF <br> JE <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 5 | 6.0 | 5 | 6.0 | 0.966 | 61.4 | LOS E | 14.2 | 372.0 | 0.64 | 1.56 | 2.85 | 12.0 |
| 8 | T1 | 1 | 6.0 | 1 | 6.0 | 0.966 | 61.4 | LOS E | 14.2 | 372.0 | 0.64 | 1.56 | 2.85 | 18.3 |
| 18 | R2 | 418 | 6.0 | 418 | 6.0 | 0.966 | 61.4 | LOS E | 14.2 | 372.0 | 0.64 | 1.56 | 2.85 | 12.0 |
| Appr | ach | 424 | 6.0 | 424 | 6.0 | 0.966 | 61.4 | LOS E | 14.2 | 372.0 | 0.64 | 1.56 | 2.85 | 12.0 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 | T1 | 555 | 7.0 | 555 | 7.0 | 0.734 | 14.8 | LOS B | 3.4 | 90.5 | 0.41 | 0.18 | 0.41 | 9.1 |
| 16 | R2 | 291 | 7.0 | 291 | 7.0 | 0.734 | 14.8 | LOS B | 3.4 | 90.5 | 0.41 | 0.18 | 0.41 | 25.0 |
| Approach |  | 846 | 7.0 | 846 | 7.0 | 0.734 | 14.8 | LOS B | 3.4 | 90.5 | 0.41 | 0.18 | 0.41 | 17.7 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 38 | 6.0 | 38 | 6.0 | 0.654 | 16.3 | LOS B | 12.9 | 337.9 | 0.00 | 0.00 | 0.00 | 35.5 |
| 2 | T1 | 379 | 6.0 | 379 | 6.0 | 0.654 | 16.3 | LOS B | 12.9 | 337.9 | 0.00 | 0.00 | 0.00 | 27.5 |
| Approach |  | 418 | 6.0 | 418 | 6.0 | 0.654 | 16.3 | LOS B | 12.9 | 337.9 | 0.00 | 0.00 | 0.00 | 29.3 |
| All Vehicles |  | 1688 | 6.5 | 1688 | 6.5 | 0.966 | 26.9 | LOS C | 14.2 | 372.0 | 0.37 | 0.48 | 0.92 | 15.8 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\nabla$ Site: 103 [Cook Rd/Old Hwy 99 N (Site Folder: Roundabout Alternative - 2028 PM Peak Hour)]

Network: N102 [2028 PM
Network (Network Folder:
General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{gathered} \text { ND } \\ \text { VS } \\ \text { HV ] } \\ \% \\ \hline \end{gathered}$ | ARR <br> FLO <br> [ Tota <br> veh/h | VAL WS HV ] \% | Deg Satn <br> v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | $\begin{aligned} & \text { 3ACK OF } \\ & \text { UE } \\ & \text { Dist ] } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 99 | 6.0 | 99 | 6.0 | 0.223 | 11.1 | LOS B | 0.3 | 8.2 | 0.60 | 0.60 | 0.60 | 23.9 |
| 8 T1 | 229 | 6.0 | 229 | 6.0 | 0.707 | 21.3 | LOS C | 17.7 | 464.0 | 0.67 | 0.94 | 1.40 | 27.7 |
| 18 R2 | 182 | 6.0 | 182 | 6.0 | 0.707 | 21.3 | LOS C | 17.7 | 464.0 | 0.67 | 0.94 | 1.40 | 21.2 |
| Approach | 510 | 6.0 | 510 | 6.0 | 0.707 | 19.4 | LOS B | 17.7 | 464.0 | 0.66 | 0.87 | 1.25 | 25.2 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 47 | 4.0 | 47 | 4.0 | 0.086 | 7.6 | LOS A | 0.1 | 3.0 | 0.46 | 0.41 | 0.46 | 28.1 |
| 6 T1 | 552 | 4.0 | 552 | 4.0 | 0.680 | 15.1 | LOS B | 1.9 | 50.0 | 0.65 | 0.81 | 1.13 | 5.9 |
| 16 R2 | 68 | 4.0 | 68 | 4.0 | 0.680 | 15.1 | LOS B | 1.9 | 50.0 | 0.65 | 0.81 | 1.13 | 24.8 |
| Approach | 667 | 4.0 | 667 | 4.0 | 0.680 | 14.6 | LOS B | 1.9 | 50.0 | 0.63 | 0.78 | 1.08 | 12.5 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 68 | 10.0 | 68 | 10.0 | 0.292 | 15.4 | LOS B | 9.6 | 260.2 | 0.63 | 0.63 | 0.63 | 22.1 |
| 4 T1 | 94 | 10.0 | 94 | 10.0 | 0.321 | 8.7 | LOS A | 0.7 | 18.1 | 0.67 | 0.64 | 0.67 | 32.7 |
| 14 R2 | 141 | 10.0 | 141 | 10.0 | 0.321 | 8.7 | LOS A | 0.7 | 18.1 | 0.67 | 0.64 | 0.67 | 27.5 |
| Approach | 302 | 10.0 | 302 | 10.0 | 0.321 | 10.2 | LOS B | 9.6 | 260.2 | 0.66 | 0.64 | 0.66 | 28.3 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 135 | 6.0 | 135 | 6.0 | 0.588 | 13.8 | LOS B | 8.6 | 225.0 | 0.41 | 0.32 | 0.44 | 25.9 |
| 2 T1 | 510 | 6.0 | 510 | 6.0 | 0.588 | 14.0 | LOS B | 8.6 | 225.0 | 0.41 | 0.33 | 0.44 | 10.0 |
| 12 R2 | 104 | 6.0 | 104 | 6.0 | 0.588 | 14.3 | LOS B | 8.6 | 225.0 | 0.41 | 0.33 | 0.45 | 25.4 |
| Approach | 750 | 6.0 | 750 | 6.0 | 0.588 | 14.0 | LOS B | 8.6 | 225.0 | 0.41 | 0.33 | 0.44 | 17.9 |
| All Vehicles | 2229 | 5.9 | 2229 | 5.9 | 0.707 | 14.9 | LOS B | 17.7 | 464.0 | 0.57 | 0.63 | 0.85 | 21.4 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing (Site Folder: Roundabout Alternative 2028 PM Peak Hour)]

Network: N102 [2028 PM
Network (Network Folder:
General)]

## New Site

Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=2900$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \end{aligned}$ | ARR FLO [ Total veh/h | $\begin{gathered} \text { VAL } \\ \text { WS } \\ \text { HV ] } \\ \% \end{gathered}$ | Deg. Satn v/c | Aver. <br> Delay <br> sec | Level of Service | AVERAG <br> [ Veh. veh | $\begin{aligned} & \text { BACK OF } \\ & \text { UE } \\ & \text { Dist ] } \\ & \mathrm{ft} \end{aligned}$ | Prop. Que | Effective Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.006 | 1169.8 | LOS F | 0.8 | 19.9 | 0.92 | 0.59 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.006 | 1169.8 | LOS F | 0.8 | 19.9 | 0.92 | 0.59 | 0.92 | 1.8 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 667 | 4.0 | 667 | 4.0 | * 0.774 | 47.9 | LOS D | 169.1 | 4362.3 | 0.32 | 0.32 | 0.32 | 16.8 |
| Approach | 667 | 4.0 | 667 | 4.0 | 0.774 | 47.9 | LOS D | 169.1 | 4362.3 | 0.32 | 0.32 | 0.32 | 16.8 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.006 | 1169.8 | LOS F | 0.8 | 19.9 | 0.92 | 0.59 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.006 | 1169.8 | LOS F | 0.8 | 19.9 | 0.92 | 0.59 | 0.92 | 1.8 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 760 | 6.0 | 760 | 6.0 | 0.242 | 22.5 | LOS C | 1.9 | 50.0 | 0.15 | 0.14 | 0.15 | 25.5 |
| Approach | 760 | 6.0 | 760 | 6.0 | 0.242 | 22.5 | LOS C | 1.9 | 50.0 | 0.15 | 0.14 | 0.15 | 25.5 |
| All Vehicles | 1429 | 5.1 | 1429 | 5.1 | 0.774 | 36.0 | LOS D | 169.1 | 4362.3 | 0.23 | 0.22 | 0.23 | 20.1 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: HCM Delay Formula (Geometric Delay is not included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)


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Project: M:I23|1.23051.00-Cook Rd and I-5 Interchange Improvements|Traffic Analysis $\operatorname{ITraffic}$ OperationsIRAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

Site: 101 [Cook Rd/I-5 SB_no RR (Site Folder: Roundabout
Alternative -2028 PM Peak Hour)]

## Network: N102 [2028 PM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | VAL <br> NS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | ACK OF <br> E <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 417 | 8.0 | 417 | 8.0 | 0.449 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.8 |
| 6 T1 | 120 | 8.0 | 120 | 8.0 | 0.449 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.1 |
| Approach | 536 | 8.0 | 536 | 8.0 | 0.449 | 7.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.9 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 193 | 6.0 | 193 | 6.0 | 0.276 | 7.6 | LOS A | 0.6 | 14.9 | 0.61 | 0.55 | 0.61 | 25.9 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.276 | 7.6 | LOS A | 0.6 | 14.9 | 0.61 | 0.55 | 0.61 | 31.2 |
| 14 R2 | 21 | 6.0 | 21 | 6.0 | 0.276 | 7.6 | LOS A | 0.6 | 14.9 | 0.61 | 0.55 | 0.61 | 30.5 |
| Approach | 219 | 6.0 | 219 | 6.0 | 0.276 | 7.6 | LOS A | 0.6 | 14.9 | 0.61 | 0.55 | 0.61 | 26.7 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 208 | 4.0 | 208 | 4.0 | 0.297 | 8.4 | LOS A | 0.7 | 17.0 | 0.68 | 0.63 | 0.68 | 27.8 |
| 12 R2 | 10 | 4.0 | 10 | 4.0 | 0.297 | 8.4 | LOS A | 0.7 | 17.0 | 0.68 | 0.63 | 0.68 | 32.0 |
| Approach | 219 | 4.0 | 219 | 4.0 | 0.297 | 8.4 | LOS A | 0.7 | 17.0 | 0.68 | 0.63 | 0.68 | 28.2 |
| All Vehicles | 974 | 6.7 | 974 | 6.7 | 0.449 | 7.8 | LOS A | 0.7 | 17.0 | 0.29 | 0.27 | 0.29 | 30.4 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D),
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## © Site: 102 [Cook Rd/I-5 NB_no RR (Site Folder: Roundabout Alternative - 2028 PM Peak Hour)]

## Network: N102 [2028 PM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \\ & \hline \end{aligned}$ | ARRIVAL FLOWS [ Total HV ] veh/h \% |  | Deg Satn v/c | Aver. Delay sec | Level of Service |  |  | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: l-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 5 | 6.0 | 5 | 6.0 | 0.487 | 10.4 | LOS B | 1.4 | 35.9 | 0.64 | 0.62 | 0.75 | 26.3 |
| 8 T1 | 1 | 6.0 | 1 | 6.0 | 0.487 | 10.4 | LOS B | 1.4 | 35.9 | 0.64 | 0.62 | 0.75 | 31.7 |
| 18 R2 | 418 | 6.0 | 418 | 6.0 | 0.487 | 10.4 | LOS B | 1.4 | 35.9 | 0.64 | 0.62 | 0.75 | 26.3 |
| Approach | 424 | 6.0 | 424 | 6.0 | 0.487 | 10.4 | LOS B | 1.4 | 35.9 | 0.64 | 0.62 | 0.75 | 26.3 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 555 | 7.0 | 555 | 7.0 | 0.734 | 14.8 | LOS B | 3.4 | 90.5 | 0.41 | 0.18 | 0.41 | 9.1 |
| 16 R2 | 291 | 7.0 | 291 | 7.0 | 0.734 | 14.8 | LOS B | 3.4 | 90.5 | 0.41 | 0.18 | 0.41 | 25.0 |
| Approach | 846 | 7.0 | 846 | 7.0 | 0.734 | 14.8 | LOS B | 3.4 | 90.5 | 0.41 | 0.18 | 0.41 | 17.7 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 38 | 6.0 | 38 | 6.0 | 0.343 | 6.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 35.5 |
| 2 T1 | 379 | 6.0 | 379 | 6.0 | 0.343 | 6.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 27.5 |
| Approach | 418 | 6.0 | 418 | 6.0 | 0.343 | 6.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 29.3 |
| All Vehicles | 1688 | 6.5 | 1688 | 6.5 | 0.734 | 11.6 | LOS B | 3.4 | 90.5 | 0.37 | 0.25 | 0.39 | 22.2 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab). Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement. LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6), Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included)
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D),
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

$\nabla$ Site: 103 [Cook Rd/Old Hwy 99 N_no RR (Site Folder: Roundabout Alternative - 2028 PM Peak Hour)]

Hetwork: N102 [2028 PM<br>Network - No Railroad (Network<br>Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID |  |  | ND WS HV] \% | ARR FLO [ Total veh/h | VAL WS HV ] \% | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | AVERAG QU [ Veh. veh | ACK OF JE Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 99 | 6.0 | 99 | 6.0 | 0.223 | 11.1 | LOS B | 0.3 | 8.2 | 0.60 | 0.60 | 0.60 | 23.9 |
| 8 | T1 | 229 | 6.0 | 229 | 6.0 | 0.489 | 10.7 | LOS B | 1.3 | 33.1 | 0.66 | 0.76 | 0.92 | 31.8 |
| 18 | R2 | 182 | 6.0 | 182 | 6.0 | 0.489 | 10.7 | LOS B | 1.3 | 33.1 | 0.66 | 0.76 | 0.92 | 26.3 |
| Appr | ach | 510 | 6.0 | 510 | 6.0 | 0.489 | 10.8 | LOS B | 1.3 | 33.1 | 0.65 | 0.73 | 0.86 | 28.9 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 47 | 4.0 | 47 | 4.0 | 0.086 | 7.6 | LOS A | 0.1 | 3.2 | 0.51 | 0.45 | 0.51 | 28.1 |
| 6 | T1 | 552 | 4.0 | 552 | 4.0 | 0.680 | 15.1 | LOS B | 1.9 | 50.0 | 0.72 | 0.90 | 1.25 | 5.9 |
| 16 | R2 | 68 | 4.0 | 68 | 4.0 | 0.680 | 15.1 | LOS B | 1.9 | 50.0 | 0.72 | 0.90 | 1.25 | 24.8 |
| Appr | ach | 667 | 4.0 | 667 | 4.0 | 0.680 | 14.6 | LOS B | 1.9 | 50.0 | 0.70 | 0.87 | 1.20 | 12.5 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 68 | 10.0 | 68 | 10.0 | 0.150 | 10.1 | LOS B | 0.2 | 6.7 | 0.63 | 0.63 | 0.63 | 24.4 |
| 4 | T1 | 94 | 10.0 | 94 | 10.0 | 0.328 | 8.9 | LOS A | 0.7 | 17.8 | 0.67 | 0.66 | 0.67 | 32.6 |
| 14 | R2 | 141 | 10.0 | 141 | 10.0 | 0.328 | 8.9 | LOS A | 0.7 | 17.8 | 0.67 | 0.66 | 0.67 | 27.3 |
| Appr | ach | 302 | 10.0 | 302 | 10.0 | 0.328 | 9.2 | LOS A | 0.7 | 17.8 | 0.66 | 0.65 | 0.66 | 28.8 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 135 | 6.0 | 135 | 6.0 | 0.349 | 6.9 | LOS A | 0.7 | 18.5 | 0.41 | 0.30 | 0.41 | 29.8 |
| 2 | T1 | 510 | 6.0 | 510 | 6.0 | 0.349 | 6.9 | LOS A | 0.7 | 18.5 | 0.41 | 0.29 | 0.41 | 14.2 |
| 12 | R2 | 104 | 6.0 | 104 | 6.0 | 0.349 | 6.8 | LOS A | 0.7 | 18.5 | 0.41 | 0.29 | 0.41 | 29.6 |
| Approach |  | 750 | 6.0 | 750 | 6.0 | 0.349 | 6.9 | LOS A | 0.7 | 18.5 | 0.41 | 0.29 | 0.41 | 22.8 |
| All V | icles | 2229 | 5.9 | 2229 | 5.9 | 0.680 | 10.4 | LOS B | 1.9 | 50.0 | 0.59 | 0.61 | 0.78 | 24.1 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6)
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing_no RR (Site Folder: Roundabout Alternative - 2028 PM Peak Hour)]

## Network: N102 [2028 PM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=61$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | DEM <br> [ Total veh/h | $\begin{aligned} & \text { VD } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | $\begin{aligned} & \text { VAL } \\ & \text { WS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> [ Veh. veh | ACK OF <br> E <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.034 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.034 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 667 | 4.0 | 667 | 4.0 | * 0.663 | 1.5 | LOS A | 1.5 | 39.8 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 667 | 4.0 | 667 | 4.0 | 0.663 | 1.5 | LOS A | 1.5 | 39.8 | 0.00 | 0.00 | 0.00 | 40.0 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.034 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.034 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.57 | 1.00 | 26.0 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 760 | 6.0 | 760 | 6.0 | 0.217 | 2.3 | LOS A | 0.4 | 9.5 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 760 | 6.0 | 760 | 6.0 | 0.217 | 2.3 | LOS A | 0.4 | 9.5 | 0.00 | 0.00 | 0.00 | 40.0 |
| All Vehicles | 1429 | 5.1 | 1429 | 5.1 | 0.663 | 2.0 | LOS A | 1.5 | 39.8 | 0.00 | 0.00 | 0.00 | 40.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: HCM Delay Formula (Geometric Delay is not included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)


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Project: M:I23|1.23051.00-Cook Rd and I-5 Interchange Improvements|Traffic Analysis $\operatorname{ITraffic}$ OperationsIRAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## $\square$ Site: 101 [Cook Rd/I-5 SB (Site Folder: Roundabout Alternative - 2045 AM Peak Hour)]

-a Network: N103 [2045 AM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | VAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> [ Veh. veh | ACK OF E Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 535 | 16.0 | 535 | 16.0 | 0.646 | 11.4 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.8 |
| 6 T1 | 255 | 16.0 | 255 | 16.0 | 0.646 | 11.4 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.2 |
| Approach | 790 | 16.0 | 790 | 16.0 | 0.646 | 11.4 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.9 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $7 \quad$ L2 | 225 | 6.0 | 225 | 6.0 | 0.482 | 13.4 | LOS B | 1.1 | 28.6 | 0.79 | 0.87 | 1.01 | 23.1 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.482 | 13.4 | LOS B | 1.1 | 28.6 | 0.79 | 0.87 | 1.01 | 29.0 |
| 14 R2 | 35 | 6.0 | 35 | 6.0 | 0.482 | 13.4 | LOS B | 1.1 | 28.6 | 0.79 | 0.87 | 1.01 | 28.4 |
| Approach | 265 | 6.0 | 265 | 6.0 | 0.482 | 13.4 | LOS B | 1.1 | 28.6 | 0.79 | 0.87 | 1.01 | 24.2 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 120 | 16.0 | 12010 | $\begin{aligned} & 16.0 \\ & 16.0 \end{aligned}$ | $\begin{aligned} & 0.281 \\ & 0.281 \end{aligned}$ | $\begin{aligned} & 10.3 \\ & 10.3 \end{aligned}$ | $\begin{aligned} & \text { LOS B } \\ & \text { LOS B } \end{aligned}$ | $\begin{aligned} & 0.5 \\ & 0.5 \end{aligned}$ | $\begin{aligned} & 13.6 \\ & 13.6 \end{aligned}$ | 0.75 | 0.73 | 0.75 | 26.6 |
| 12 R2 | 10 | 16.0 |  |  |  |  |  |  |  | 0.75 | 0.73 | 0.75 | 30.8 |
| Approach | 130 | 16.0 | 130 | 16.0 | 0.281 | 10.3 | LOS B | 0.5 | 13.6 | 0.75 | 0.73 | 0.75 | 27.1 |
| All Vehicles | 1185 | 13.8 | 1185 | 13.8 | 0.646 | 11.7 | LOS B | 1.1 | 28.6 | 0.26 | 0.27 | 0.31 | 30.1 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## $\forall$ Site: 102 [Cook Rd/I-5 NB (Site Folder: Roundabout Alternative - 2045 AM Peak Hour)]

마 Network: N103 [2045 AM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{ll} \text { Mov Turn } \\ \text { ID } \end{array}$ |  | $\begin{aligned} & \text { VD } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | VAL WS HV ] \% | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service | AVERA Q <br> [ Veh. veh | BACK OF JE Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L 2 | 1 | 15.0 | 1 | 15.0 | 0.531 | 14.8 | LOS B | 4.9 | 136.8 | 0.50 | 0.50 | 0.64 | 23.8 |
| 8 T1 | 5 | 15.0 | 5 | 15.0 | 0.531 | 14.8 | LOS B | 4.9 | 136.8 | 0.50 | 0.50 | 0.64 | 29.7 |
| 18 R2 | 245 | 15.0 | 245 | 15.0 | 0.531 | 14.8 | LOS B | 4.9 | 136.8 | 0.50 | 0.50 | 0.64 | 23.8 |
| Approach | 251 | 15.0 | 251 | 15.0 | 0.531 | 14.8 | LOS B | 4.9 | 136.8 | 0.50 | 0.50 | 0.64 | 24.0 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $6 \quad$ T1 | 790 | 12.0 | 790 | 12.0 | 0.827 | 19.0 | LOS B | 6.1 | 167.7 | 0.35 | 0.11 | 0.35 | 7.9 |
| 16 R2 | 240 | 12.0 | 240 | 12.0 | 0.827 | 19.0 | LOS B | 6.1 | 167.7 | 0.35 | 0.11 | 0.35 | 23.1 |
| Approach | 1030 | 12.0 | 1030 | 12.0 | 0.827 | 19.0 | LOS B | 6.1 | 167.7 | 0.35 | 0.11 | 0.35 | 13.7 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 10 | 8.0 | 10 | 8.0 | 0.525 | 11.2 | LOS B | 6.8 | 180.5 | 0.00 | 0.00 | 0.00 | 35.6 |
| $2 \quad \mathrm{~T} 1$ | 340 | 8.0 | 340 | 8.0 | 0.525 | 11.2 | LOS B | 6.8 | 180.5 | 0.00 | 0.00 | 0.00 | 28.0 |
| Approach | 350 | 8.0 | 350 | 8.0 | 0.525 | 11.2 | LOS B | 6.8 | 180.5 | 0.00 | 0.00 | 0.00 | 28.6 |
| All Vehicles | 1631 | 11.6 | 1631 | 11.6 | 0.827 | 16.6 | LOS B | 6.8 | 180.5 | 0.30 | 0.14 | 0.32 | 17.6 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\square$ Site: 103 [Cook Rd/OId Hwy 99 N (Site Folder: Roundabout Alternative - 2045 AM Peak Hour)]

마 Network: N103 [2045 AM
Network (Network Folder:
General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | IVAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> [ Veh. veh | ACK OF <br> JE <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 110 | 22.0 | 110 | 22.0 | 0.215 | 8.0 | LOS A | 0.2 | 7.1 | 0.53 | 0.51 | 0.53 | 25.5 |
| 8 T1 | 70 | 22.0 | 70 | 22.0 | 0.239 | 7.5 | LOS A | 6.4 | 188.3 | 0.52 | 0.49 | 0.52 | 33.0 |
| 18 R2 | 65 | 22.0 | 65 | 22.0 | 0.239 | 7.5 | LOS A | 6.4 | 188.3 | 0.52 | 0.49 | 0.52 | 28.4 |
| Approach | 245 | 22.0 | 245 | 22.0 | 0.239 | 7.7 | LOS A | 6.4 | 188.3 | 0.53 | 0.50 | 0.53 | 28.9 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 140 | 9.0 | 140 | 9.0 | 0.202 | 7.5 | LOS A | 0.3 | 7.6 | 0.40 | 0.34 | 0.40 | 28.0 |
| $6 \quad$ T1 | 670 | 9.0 | 670 | 9.0 | 0.870 | 29.0 | LOS C | 1.9 | 50.0 | 0.53 | 0.95 | 1.58 | 3.5 |
| 16 R2 | 45 | 9.0 | 45 | 9.0 | 0.870 | 29.0 | LOS C | 1.9 | 50.0 | 0.53 | 0.95 | 1.58 | 19.3 |
| Approach | 855 | 9.0 | 855 | 9.0 | 0.870 | 25.5 | LOS C | 1.9 | 50.0 | 0.51 | 0.85 | 1.39 | 9.8 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 65 | 21.0 | 65 | 21.0 | 0.309 | 17.5 | LOS B | 7.4 | 215.2 | 0.69 | 0.70 | 0.72 | 21.3 |
| 4 T1 | 215 | 21.0 | 215 | 21.0 | 0.814 | 31.6 | LOS C | 3.8 | 110.8 | 0.90 | 1.34 | 2.04 | 24.4 |
| 14 R2 | 240 | 21.0 | 240 | 21.0 | 0.814 | 31.6 | LOS C | 3.8 | 110.8 | 0.90 | 1.34 | 2.04 | 17.8 |
| Approach | 520 | 21.0 | 520 | 21.0 | 0.814 | 29.8 | LOS C | 7.4 | 215.2 | 0.87 | 1.26 | 1.88 | 21.5 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 100 | 13.0 | 100 | 13.0 | 0.537 | 14.7 | LOS B | 8.2 | 225.0 | 0.57 | 0.64 | 0.82 | 25.2 |
| 2 T1 | 380 | 13.0 | 380 | 13.0 | 0.537 | 14.0 | LOS B | 8.2 | 225.0 | 0.57 | 0.62 | 0.79 | 10.0 |
| 12 R 2 | 115 | 13.0 | 115 | 13.0 | 0.537 | 13.3 | LOS B | 8.2 | 225.0 | 0.56 | 0.60 | 0.77 | 25.7 |
| Approach | 595 | 13.0 | 595 | 13.0 | 0.537 | 13.9 | LOS B | 8.2 | 225.0 | 0.57 | 0.62 | 0.79 | 18.5 |
| All Vehicles | 2215 | 14.3 | 2215 | 14.3 | 0.870 | 21.4 | LOS C | 8.2 | 225.0 | 0.61 | 0.85 | 1.25 | 18.2 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing (Site Folder: Roundabout Alternative 2045 AM Peak Hour)]

무 Network: N103 [2045 AM
Network (Network Folder:
General)]

## New Site

Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=2900$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{gathered} \text { ND } \\ \text { VS } \\ \text { HV ] } \\ \% \end{gathered}$ | ARR FLO [ Tota veh/h | IVAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG Q [ Veh. veh | $\begin{aligned} & \text { BACK OF } \\ & \text { UE } \\ & \text { Dist ] } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 855 | 9.0 | 855 | 9.0 | * 0.924 | 73.7 | LOS E | 282.7 | 7575.9 | 0.45 | 0.44 | 0.45 | 12.8 |
| Approach | 855 | 9.0 | 855 | 9.0 | 0.924 | 73.7 | LOS E | 282.7 | 7575.9 | 0.45 | 0.44 | 0.45 | 12.8 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 510 | 13.0 | 510 | 13.0 | 0.173 | 20.5 | LOS C | 1.8 | 50.0 | 0.14 | 0.13 | 0.14 | 26.2 |
| Approach | 510 | 13.0 | 510 | 13.0 | 0.173 | 20.5 | LOS C | 1.8 | 50.0 | 0.14 | 0.13 | 0.14 | 26.2 |
| All Vehicles | 1367 | 10.5 | 1367 | 10.5 | 0.924 | 55.5 | LOS E | 282.7 | 7575.9 | 0.33 | 0.32 | 0.33 | 15.6 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: HCM Delay Formula (Geometric Delay is not included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)


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Project: M:I23|1.23051.00-Cook Rd and I-5 Interchange Improvements|Traffic Analysis $\operatorname{ITraffic}$ OperationsIRAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## - Site: 101 [Cook Rd/I-5 SB_no RR (Site Folder: Roundabout Alternative - 2045 AM Peak Hour)]

무 Network: N103 [2045 AM Network - No Railroad (Network

Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{gathered} \text { ND } \\ \text { VS } \\ \text { HV ] } \\ \% \end{gathered}$ | ARR FLO [ Tota veh/h | IVAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG Q [ Veh. veh | ACK OF <br> E <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| L2 | 535 | 16.0 | 535 | 16.0 | 0.646 | 11.4 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.8 |
| 6 T1 | 255 | 16.0 | 255 | 16.0 | 0.646 | 11.4 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.2 |
| Approach | 790 | 16.0 | 790 | 16.0 | 0.646 | 11.4 | LOS B | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 32.9 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 225 | 6.0 | 225 | 6.0 | 0.373 | 9.9 | LOS A | 0.9 | 24.5 | 0.79 | 0.78 | 0.81 | 24.7 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.373 | 9.9 | LOS A | 0.9 | 24.5 | 0.79 | 0.78 | 0.81 | 30.3 |
| 14 R2 | 35 | 6.0 | 35 | 6.0 | 0.373 | 9.9 | LOS A | 0.9 | 24.5 | 0.79 | 0.78 | 0.81 | 29.6 |
| Approach | 265 | 6.0 | 265 | 6.0 | 0.373 | 9.9 | LOS A | 0.9 | 24.5 | 0.79 | 0.78 | 0.81 | 25.8 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 120 | 16.0 | 120 | 16.0 | 0.214 | 8.6 | LOS A | 0.5 | 13.6 | 0.75 | 0.71 | 0.75 | 27.7 |
| 12 R2 | 10 | 16.0 | 10 | 16.0 | 0.214 | 8.6 | LOS A | 0.5 | 13.6 | 0.75 | 0.71 | 0.75 | 31.6 |
| Approach | 130 | 16.0 | 130 | 16.0 | 0.214 | 8.6 | LOS A | 0.5 | 13.6 | 0.75 | 0.71 | 0.75 | 28.2 |
| All Vehicles | 1185 | 13.8 | 1185 | 13.8 | 0.646 | 10.7 | LOS B | 0.9 | 24.5 | 0.26 | 0.25 | 0.26 | 30.7 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## © Site: 102 [Cook Rd/I-5 NB_no RR (Site Folder: Roundabout Alternative - 2045 AM Peak Hour)]

무 Network: N103 [2045 AM Network - No Railroad (Network

Folder: General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | $\begin{aligned} & \text { VD } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARRIVAL FLOWS [ Total HV ] veh/h \% |  | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> Q <br> [ Veh. veh | ACK OF <br> JE <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L 2 | 1 | 15.0 | 1 | 15.0 | 0.269 | 6.6 | LOS A | 0.5 | 15.1 | 0.50 | 0.40 | 0.50 | 28.9 |
| 8 T1 | 5 | 15.0 | 5 | 15.0 | 0.269 | 6.6 | LOS A | 0.5 | 15.1 | 0.50 | 0.40 | 0.50 | 33.4 |
| 18 R2 | 245 | 15.0 | 245 | 15.0 | 0.269 | 6.6 | LOS A | 0.5 | 15.1 | 0.50 | 0.40 | 0.50 | 28.9 |
| Approach | 251 | 15.0 | 251 | 15.0 | 0.269 | 6.6 | LOS A | 0.5 | 15.1 | 0.50 | 0.40 | 0.50 | 29.0 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $6 \quad$ T1 | 790 | 12.0 | 790 | 12.0 | 0.827 | 19.0 | LOS B | 6.1 | 167.7 | 0.35 | 0.11 | 0.35 | 7.9 |
| 16 R2 | 240 | 12.0 | 240 | 12.0 | 0.827 | 19.0 | LOS B | 6.1 | 167.7 | 0.35 | 0.11 | 0.35 | 23.1 |
| Approach | 1030 | 12.0 | 1030 | 12.0 | 0.827 | 19.0 | LOS B | 6.1 | 167.7 | 0.35 | 0.11 | 0.35 | 13.7 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 10 | 8.0 | 10 | 8.0 | 0.266 | 5.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 35.6 |
| $2 \quad \mathrm{~T} 1$ | 340 | 8.0 | 340 | 8.0 | 0.266 | 5.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 28.0 |
| Approach | 350 | 8.0 | 350 | 8.0 | 0.266 | 5.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 28.6 |
| All Vehicles | 1631 | 11.6 | 1631 | 11.6 | 0.827 | 14.1 | LOS B | 6.1 | 167.7 | 0.30 | 0.13 | 0.30 | 18.3 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

$\nabla$ Site: 103 [Cook Rd/Old Hwy 99 N_no RR (Site Folder: Roundabout Alternative - 2045 AM Peak Hour)]

마 Network: N103 [2045 AM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | IVAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | ACK OF E Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 110 | 22.0 | 110 | 22.0 | 0.215 | 8.0 | LOS A | 0.2 | 7.1 | 0.53 | 0.51 | 0.53 | 25.5 |
| 8 T1 | 70 | 22.0 | 70 | 22.0 | 0.161 | 5.9 | LOS A | 0.3 | 7.7 | 0.52 | 0.47 | 0.52 | 33.8 |
| 18 R2 | 65 | 22.0 | 65 | 22.0 | 0.161 | 5.9 | LOS A | 0.3 | 7.7 | 0.52 | 0.47 | 0.52 | 29.5 |
| Approach | 245 | 22.0 | 245 | 22.0 | 0.215 | 6.9 | LOS A | 0.3 | 7.7 | 0.53 | 0.49 | 0.53 | 29.4 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 140 | 9.0 | 140 | 9.0 | 0.202 | 7.5 | LOS A | 0.3 | 8.4 | 0.45 | 0.39 | 0.45 | 28.0 |
| 6 T1 | 670 | 9.0 | 670 | 9.0 | 0.870 | 29.0 | LOS C | 1.9 | 50.0 | 0.60 | 1.08 | 1.78 | 3.5 |
| 16 R2 | 45 | 9.0 | 45 | 9.0 | 0.870 | 29.0 | LOS C | 1.9 | 50.0 | 0.60 | 1.08 | 1.78 | 19.3 |
| Approach | 855 | 9.0 | 855 | 9.0 | 0.870 | 25.5 | LOS C | 1.9 | 50.0 | 0.58 | 0.97 | 1.56 | 9.8 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 65 | 21.0 | 65 | 21.0 | 0.165 | 11.7 | LOS B | 0.3 | 7.9 | 0.69 | 0.69 | 0.69 | 23.7 |
| 4 T1 | 215 | 21.0 | 215 | 21.0 | 0.844 | 36.1 | LOS D | 3.9 | 113.4 | 0.88 | 1.38 | 2.26 | 23.3 |
| 14 R2 | 240 | 21.0 | 240 | 21.0 | 0.844 | 36.1 | LOS D | 3.9 | 113.4 | 0.88 | 1.38 | 2.26 | 16.6 |
| Approach | 520 | 21.0 | 520 | 21.0 | 0.844 | 33.0 | LOS C | 3.9 | 113.4 | 0.86 | 1.30 | 2.06 | 20.6 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 100 | 13.0 | 100 | 13.0 | 0.328 | 7.9 | LOS A | 0.6 | 17.8 | 0.57 | 0.51 | 0.57 | 28.9 |
| 2 T1 | 380 | 13.0 | 380 | 13.0 | 0.328 | 7.5 | LOS A | 0.7 | 18.4 | 0.56 | 0.50 | 0.56 | 13.6 |
| 12 R2 | 115 | 13.0 | 115 | 13.0 | 0.328 | 7.2 | LOS A | 0.7 | 18.4 | 0.56 | 0.49 | 0.56 | 29.1 |
| Approach | 595 | 13.0 | 595 | 13.0 | 0.328 | 7.5 | LOS A | 0.7 | 18.4 | 0.56 | 0.50 | 0.56 | 22.8 |
| All Vehicles | 2215 | 14.3 | 2215 | 14.3 | 0.870 | 20.4 | LOS C | 3.9 | 113.4 | 0.63 | 0.87 | 1.30 | 18.6 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6)
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing _no RR (Site Folder: Roundabout Alternative - 2045 AM Peak Hour)]

Network: N103 [2045 AM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=61$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | $\begin{gathered} \text { ND } \\ \text { VS } \\ \text { HV ] } \\ \% \end{gathered}$ | ARR FLO [ Total veh/h | VAL WS HV ] \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> [ Veh. veh | ACK OF E Dist ] ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 855 | 9.0 | 855 | 9.0 | * 0.766 | 3.1 | LOS A | 2.4 | 64.8 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 855 | 9.0 | 855 | 9.0 | 0.766 | 3.1 | LOS A | 2.4 | 64.8 | 0.00 | 0.00 | 0.00 | 40.0 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2 T1 | 510 | 13.0 | 510 | 13.0 | 0.155 | 1.8 | LOS A | 0.2 | 6.3 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 510 | 13.0 | 510 | 13.0 | 0.155 | 1.8 | LOS A | 0.2 | 6.3 | 0.00 | 0.00 | 0.00 | 40.0 |
| All Vehicles | 1367 | 10.5 | 1367 | 10.5 | 0.766 | 2.6 | LOS A | 2.4 | 64.8 | 0.00 | 0.00 | 0.00 | 40.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: HCM Delay Formula (Geometric Delay is not included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)


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Project: M:I23|1.23051.00-Cook Rd and I-5 Interchange Improvements|Traffic Analysis $\operatorname{ITraffic}$ OperationsIRAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## $\forall$ Site: 101 [Cook Rd/I-5 SB (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

Network: N101 [2045 PM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{\|l\|} \hline \text { Mov } \\ \hline \text { ID } \end{array}$ |  | DEM <br> [ Total veh/h | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \end{aligned}$ | ARR FLO [ Tota veh/h | VAL <br> NS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG <br> [ Veh. veh | $\begin{aligned} & \text { ACK OF } \\ & \text { JE } \\ & \text { Dist ] } \\ & \text { ft } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 405 | 8.0 | 405 | 8.0 | 0.415 | 6.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.0 |
| 6 | T1 | 140 | 8.0 | 140 | 8.0 | 0.415 | 6.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.2 |
| App | ach | 545 | 8.0 | 545 | 8.0 | 0.415 | 6.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.0 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 190 | 6.0 | 190 | 6.0 | 0.453 | 12.2 | LOS B | 0.6 | 14.7 | 0.59 | 0.58 | 0.67 | 23.6 |
| 4 | T1 | 5 | 6.0 | 5 | 6.0 | 0.453 | 12.2 | LOS B | 0.6 | 14.7 | 0.59 | 0.58 | 0.67 | 29.4 |
| 14 | R2 | 25 | 6.0 | 25 | 6.0 | 0.453 | 12.2 | LOS B | 0.6 | 14.7 | 0.59 | 0.58 | 0.67 | 28.8 |
| Approach |  | 220 | 6.0 | 220 | 6.0 | 0.453 | 12.2 | LOS B | 0.6 | 14.7 | 0.59 | 0.58 | 0.67 | 24.6 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & 2 \\ & 12 \end{aligned}$ | T1 | 370 | 4.0 | 370 | 4.0 | 0.870 | 43.2 | LOS D | 3.2 | 81.8 | 0.73 | 1.30 | 2.17 | 15.2 |
|  | R2 | 10 | 4.0 | 10 | 4.0 | 0.870 | 43.2 | LOS D | 3.2 | 81.8 | 0.73 | 1.30 | 2.17 | 21.4 |
| Approach |  | 380 | 4.0 | 380 | 4.0 | 0.870 | 43.2 | LOS D | 3.2 | 81.8 | 0.73 | 1.30 | 2.17 | 15.4 |
| All Vehicles |  | 1145 | 6.3 | 1145 | 6.3 | 0.870 | 19.9 | LOS B | 3.2 | 81.8 | 0.36 | 0.54 | 0.85 | 23.8 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00 - Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## $\forall$ Site: 102 [Cook Rd/I-5 NB (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

미 Network: N101 [2045 PM Network (Network Folder:

General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{array}{\|l\|} \hline \text { Mov } \\ \hline \text { ID } \\ \hline \end{array}$ |  |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \\ & \hline \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | VAL <br> NS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service | AVERAC <br> [ Veh. veh | ACK OF <br> JE <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: I-5 NB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 5 | 6.0 | 5 | 6.0 | 0.848 | 38.7 | LOS D | 14.1 | 370.4 | 0.67 | 1.18 | 1.95 | 15.8 |
| 8 | T1 | 1 | 6.0 | 1 | 6.0 | 0.848 | 38.7 | LOS D | 14.1 | 370.4 | 0.67 | 1.18 | 1.95 | 22.6 |
| 18 | R2 | 380 | 6.0 | 380 | 6.0 | 0.848 | 38.7 | LOS D | 14.1 | 370.4 | 0.67 | 1.18 | 1.95 | 15.8 |
| Appr | ach | 386 | 6.0 | 386 | 6.0 | 0.848 | 38.7 | LOS D | 14.1 | 370.4 | 0.67 | 1.18 | 1.95 | 15.8 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 | T1 | 535 | 7.0 | 535 | 7.0 | 0.634 | 10.8 | LOS B | 2.3 | 60.1 | 0.32 | 0.14 | 0.32 | 11.0 |
| 16 | R2 | 270 | 7.0 | 270 | 7.0 | 0.634 | 10.8 | LOS B | 2.3 | 60.1 | 0.32 | 0.14 | 0.32 | 27.1 |
| Approach |  | 805 | 7.0 | 805 | 7.0 | 0.634 | 10.8 | LOS B | 2.3 | 60.1 | 0.32 | 0.14 | 0.32 | 20.0 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 45 | 6.0 | 45 | 6.0 | 0.802 | 24.2 | LOS C | 19.1 | 500.0 | 0.00 | 0.00 | 0.00 | 35.5 |
| 2 | T1 | 515 | 6.0 | 515 | 6.0 | 0.802 | 24.2 | LOS C | 19.1 | 500.0 | 0.00 | 0.00 | 0.00 | 27.6 |
| Approach |  | 560 | 6.0 | 560 | 6.0 | 0.802 | 24.2 | LOS C | 19.1 | 500.0 | 0.00 | 0.00 | 0.00 | 29.2 |
| All Vehicles |  | 1751 | 6.5 | 1751 | 6.5 | 0.848 | 21.2 | LOS C | 19.1 | 500.0 | 0.30 | 0.33 | 0.58 | 19.6 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: M:\23\1.23051.00-Cook Rd and I-5 Interchange Improvements\Traffic Analysis\Traffic Operations\RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

$\nabla$ Site: 103 [Cook Rd/Old Hwy 99 N (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

Network: N101 [2045 PM
Network (Network Folder:
General)]
New Site
Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Tota veh/h | $\begin{aligned} & \text { VAL } \\ & \text { WS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service | $\begin{gathered} \text { AVERAG } \\ \text { Q } \\ \text { [ Veh. } \\ \text { veh } \end{gathered}$ | $\begin{aligned} & \text { 3ACK OF } \\ & \text { UE } \\ & \text { Dist ] } \\ & \text { ft } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed <br> mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 | L2 | 95 | 6.0 | 95 | 6.0 | 0.171 | 8.6 | LOS A | 0.3 | 7.0 | 0.61 | 0.61 | 0.61 | 25.1 |
| 8 | T1 | 320 | 6.0 | 320 | 6.0 | 0.826 | 28.9 | LOS C | 21.9 | 575.1 | 0.75 | 1.21 | 2.00 | 25.3 |
| 18 | R2 | 215 | 6.0 | 215 | 6.0 | 0.826 | 28.9 | LOS C | 21.9 | 575.1 | 0.75 | 1.21 | 2.00 | 18.6 |
| Appr | ach | 630 | 6.0 | 630 | 6.0 | 0.826 | 25.9 | LOS C | 21.9 | 575.1 | 0.73 | 1.12 | 1.79 | 23.4 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L2 | 55 | 4.0 | 55 | 4.0 | 0.091 | 7.0 | LOS A | 0.1 | 3.5 | 0.51 | 0.46 | 0.51 | 28.5 |
| 6 | T1 | 550 | 4.0 | 550 | 4.0 | 0.624 | 12.6 | LOS B | 1.9 | 50.0 | 0.68 | 0.82 | 1.07 | 6.7 |
| 16 | R2 | 65 | 4.0 | 65 | 4.0 | 0.624 | 12.6 | LOS B | 1.9 | 50.0 | 0.68 | 0.82 | 1.07 | 26.2 |
| Appr |  | 670 | 4.0 | 670 | 4.0 | 0.624 | 12.2 | LOS B | 1.9 | 50.0 | 0.67 | 0.79 | 1.02 | 14.1 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L2 | 70 | 10.0 | 70 | 10.0 | 0.253 | 12.1 | LOS B | 10.5 | 282.5 | 0.62 | 0.62 | 0.62 | 23.5 |
| 4 | T1 | 95 | 10.0 | 95 | 10.0 | 0.295 | 7.4 | LOS A | 0.7 | 17.8 | 0.67 | 0.61 | 0.67 | 33.3 |
| 14 | R2 | 160 | 10.0 | 160 | 10.0 | 0.295 | 7.4 | LOS A | 0.7 | 17.8 | 0.67 | 0.61 | 0.67 | 28.4 |
| Appr | ach | 325 | 10.0 | 325 | 10.0 | 0.295 | 8.4 | LOS A | 10.5 | 282.5 | 0.66 | 0.61 | 0.66 | 29.2 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 | L2 | 215 | 6.0 | 215 | 6.0 | 0.639 | 14.0 | LOS B | 8.6 | 225.0 | 0.43 | 0.37 | 0.52 | 25.6 |
| 2 | T1 | 580 | 6.0 | 580 | 6.0 | 0.639 | 15.0 | LOS B | 8.6 | 225.0 | 0.43 | 0.39 | 0.55 | 9.6 |
| 12 | R2 | 100 | 6.0 | 100 | 6.0 | 0.639 | 16.0 | LOS B | 8.6 | 225.0 | 0.42 | 0.41 | 0.58 | 24.6 |
| Approach |  | 895 | 6.0 | 895 | 6.0 | 0.639 | 14.9 | LOS B | 8.6 | 225.0 | 0.43 | 0.39 | 0.55 | 17.9 |
| All Vehicles |  | 2520 | 6.0 | 2520 | 6.0 | 0.826 | 16.1 | LOS B | 21.9 | 575.1 | 0.60 | 0.70 | 1.00 | 21.4 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing (Site Folder: Roundabout Alternative 2045 PM Peak Hour)]

Network: N101 [2045 PM
Network (Network Folder:
General)]

## New Site

Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=2900$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Total veh/h | VAL <br> WS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERAG Q [ Veh. veh | $\begin{aligned} & \text { BACK OF } \\ & \text { UE } \\ & \text { Dist ] } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 670 | 4.0 | 670 | 4.0 | * 0.778 | 48.5 | LOS D | 171.5 | 4424.4 | 0.33 | 0.32 | 0.33 | 16.7 |
| Approach | 670 | 4.0 | 670 | 4.0 | 0.778 | 48.5 | LOS D | 171.5 | 4424.4 | 0.33 | 0.32 | 0.33 | 16.7 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.005 | 1169.7 | LOS F | 0.7 | 19.1 | 0.92 | 0.58 | 0.92 | 1.8 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 865 | 6.0 | 865 | 6.0 | 0.270 | 20.9 | LOS C | 1.9 | 50.0 | 0.15 | 0.15 | 0.15 | 25.2 |
| Approach | 865 | 6.0 | 865 | 6.0 | 0.270 | 20.9 | LOS C | 1.9 | 50.0 | 0.15 | 0.15 | 0.15 | 25.2 |
| All Vehicles | 1537 | 5.1 | 1537 | 5.1 | 0.778 | 34.4 | LOS C | 171.5 | 4424.4 | 0.23 | 0.22 | 0.23 | 20.2 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: HCM Delay Formula (Geometric Delay is not included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)


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Project: M:I23|1.23051.00-Cook Rd and I-5 Interchange Improvements|Traffic Analysis|Traffic Operations|RAB Alternative_with railroad_added EBT.sip9

## MOVEMENT SUMMARY

## S Site: 101 [Cook Rd/l-5 SB_no RR (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

## Network: N101 [2045 PM Network - No Railroad (Network Folder: General)]

## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \end{aligned}$ | ARR <br> FLO <br> [ Tota <br> veh/h | VAL <br> NS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | $\begin{aligned} & \text { ACK OF } \\ & \text { JE } \\ & \text { Dist ] } \\ & \text { ft } \end{aligned}$ | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 405 | 8.0 | 405 | 8.0 | 0.415 | 6.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.0 |
| 6 T1 | 140 | 8.0 | 140 | 8.0 | 0.415 | 6.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.2 |
| Approach | 545 | 8.0 | 545 | 8.0 | 0.415 | 6.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 33.0 |
| North: I-5 SB Off Ramp |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 190 | 6.0 | 190 | 6.0 | 0.243 | 6.5 | LOS A | 0.5 | 13.3 | 0.59 | 0.51 | 0.59 | 26.6 |
| 4 T1 | 5 | 6.0 | 5 | 6.0 | 0.243 | 6.5 | LOS A | 0.5 | 13.3 | 0.59 | 0.51 | 0.59 | 31.7 |
| 14 R2 | 25 | 6.0 | 25 | 6.0 | 0.243 | 6.5 | LOS A | 0.5 | 13.3 | 0.59 | 0.51 | 0.59 | 31.0 |
| Approach | 220 | 6.0 | 220 | 6.0 | 0.243 | 6.5 | LOS A | 0.5 | 13.3 | 0.59 | 0.51 | 0.59 | 27.5 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 370 | 4.0 | 370 | 4.0 | 0.441 | 9.6 | LOS A | 1.2 | 31.4 | 0.73 | 0.70 | 0.80 | 27.1 |
| 12 R2 | 10 | 4.0 | 10 | 4.0 | 0.441 | 9.6 | LOS A | 1.2 | 31.4 | 0.73 | 0.70 | 0.80 | 31.4 |
| Approach | 380 | 4.0 | 380 | 4.0 | 0.441 | 9.6 | LOS A | 1.2 | 31.4 | 0.73 | 0.70 | 0.80 | 27.2 |
| All Vehicles | 1145 | 6.3 | 1145 | 6.3 | 0.441 | 7.6 | LOS A | 1.2 | 31.4 | 0.36 | 0.33 | 0.38 | 30.1 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included)
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

7 Site: 102 [Cook Rd/I-5 NB_no RR (Site Folder: Roundabout
Alternative - 2045 PM Peak Hour)]

## Network: N101 [2045 PM Network - No Railroad (Network

Folder: General)]

## New Site

Site Category: (None)
Roundabout


Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## MOVEMENT SUMMARY

$\nabla$ Site: 103 [Cook Rd/Old Hwy 99 N_no RR (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

\author{

+ Network: N101 [2045 PM <br> Network - No Railroad (Network <br> Folder: General)]
}


## New Site

Site Category: (None)
Roundabout

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | $\begin{gathered} \text { DEM } \\ \text { FLO } \\ \text { [ Total } \\ \text { veh/h } \end{gathered}$ | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR FLO [ Total veh/h | VAL <br> NS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | ACK OF <br> E <br> Dist ] <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3 L2 | 95 | 6.0 | 95 | 6.0 | 0.170 | 8.6 | LOS A | 0.3 | 6.7 | 0.60 | 0.60 | 0.60 | 25.2 |
| 8 T1 | 320 | 6.0 | 320 | 6.0 | 0.582 | 12.1 | LOS B | 1.9 | 48.8 | 0.73 | 0.88 | 1.15 | 31.2 |
| 18 R2 | 215 | 6.0 | 215 | 6.0 | 0.582 | 12.1 | LOS B | 1.9 | 48.8 | 0.73 | 0.88 | 1.15 | 25.5 |
| Approach | 630 | 6.0 | 630 | 6.0 | 0.582 | 11.6 | LOS B | 1.9 | 48.8 | 0.71 | 0.84 | 1.07 | 29.0 |
| East: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 55 | 4.0 | 55 | 4.0 | 0.091 | 7.0 | LOS A | 0.1 | 3.7 | 0.56 | 0.51 | 0.56 | 28.5 |
| 6 T1 | 550 | 4.0 | 550 | 4.0 | 0.624 | 12.6 | LOS B | 1.9 | 50.0 | 0.76 | 0.91 | 1.18 | 6.7 |
| 16 R2 | 65 | 4.0 | 65 | 4.0 | 0.624 | 12.6 | LOS B | 1.9 | 50.0 | 0.76 | 0.91 | 1.18 | 26.2 |
| Approach | 670 | 4.0 | 670 | 4.0 | 0.624 | 12.1 | LOS B | 1.9 | 50.0 | 0.74 | 0.87 | 1.13 | 14.1 |
| North: Old Hwy 99 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 70 | 10.0 | 70 | 10.0 | 0.130 | 8.3 | LOS A | 0.2 | 6.0 | 0.62 | 0.60 | 0.62 | 25.3 |
| 4 T1 | 95 | 10.0 | 95 | 10.0 | 0.300 | 7.5 | LOS A | 0.7 | 17.6 | 0.67 | 0.62 | 0.67 | 33.2 |
| 14 R2 | 160 | 10.0 | 160 | 10.0 | 0.300 | 7.5 | LOS A | 0.7 | 17.6 | 0.67 | 0.62 | 0.67 | 28.3 |
| Approach | 325 | 10.0 | 325 | 10.0 | 0.300 | 7.7 | LOS A | 0.7 | 17.6 | 0.66 | 0.62 | 0.66 | 29.6 |
| West: Cook Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 5 L2 | 215 | 6.0 | 215 | 6.0 | 0.386 | 7.2 | LOS A | 0.8 | 21.2 | 0.43 | 0.31 | 0.43 | 29.2 |
| 2 T1 | 580 | 6.0 | 580 | 6.0 | 0.386 | 6.9 | LOS A | 0.8 | 21.5 | 0.42 | 0.30 | 0.42 | 14.1 |
| 12 R2 | 100 | 6.0 | 100 | 6.0 | 0.386 | 6.8 | LOS A | 0.8 | 21.5 | 0.42 | 0.30 | 0.42 | 29.7 |
| Approach | 895 | 6.0 | 895 | 6.0 | 0.386 | 7.0 | LOS A | 0.8 | 21.5 | 0.42 | 0.30 | 0.42 | 23.1 |
| All Vehicles | 2520 | 6.0 | 2520 | 6.0 | 0.624 | 9.6 | LOS A | 1.9 | 50.0 | 0.61 | 0.63 | 0.80 | 25.1 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Roundabout LOS Method: Same as Signalised Intersections.
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Roundabout Capacity Model: SIDRA Standard.
Delay Model: SIDRA Standard (Geometric Delay is included).
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

## MOVEMENT SUMMARY

Site: 101 [RR Crossing_no RR (Site Folder: Roundabout Alternative - 2045 PM Peak Hour)]

## Network: N101 [2045 PM Network - No Railroad (Network Folder: General)]

New Site
Site Category: (None)
Signals - EQUISAT (Pretimed) Isolated Cycle Time $=61$ seconds (Site User-Given Phase Times)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { IS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | ARR <br> FLO <br> [ Total <br> veh/h | VAL <br> NS <br> HV ] <br> \% | Deg. Satn v/c | Aver. Delay sec | Level of Service | AVERA <br> [ Veh. veh | ACK OF <br> E <br> Dist $]$ <br> ft | Prop. Que | Effective A Stop Rate | ver. No. Cycles | Aver. Speed mph |
| South: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8 T1 | 1 | 3.0 | 1 | 3.0 | * 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| East: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6 T1 | 670 | 4.0 | 670 | 4.0 | * 0.666 | 1.6 | LOS A | 1.6 | 40.4 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 670 | 4.0 | 670 | 4.0 | 0.666 | 1.6 | LOS A | 1.6 | 40.4 | 0.00 | 0.00 | 0.00 | 40.0 |
| North: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 T1 | 1 | 3.0 | 1 | 3.0 | 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| Approach | 1 | 3.0 | 1 | 3.0 | 0.033 | 29.9 | LOS C | 0.0 | 0.5 | 1.00 | 0.56 | 1.00 | 26.0 |
| West: RoadName |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $2 \quad$ T1 | 865 | 6.0 | 865 | 6.0 | 0.246 | 2.6 | LOS A | 0.4 | 11.2 | 0.00 | 0.00 | 0.00 | 40.0 |
| Approach | 865 | 6.0 | 865 | 6.0 | 0.246 | 2.6 | LOS A | 0.4 | 11.2 | 0.00 | 0.00 | 0.00 | 40.0 |
| All Vehicles | 1537 | 5.1 | 1537 | 5.1 | 0.666 | 2.2 | LOS A | 1.6 | 40.4 | 0.00 | 0.00 | 0.00 | 40.0 |

Site Level of Service (LOS) Method: Delay \& v/c (HCM 6). Site LOS Method is specified in the Network Data dialog (Network tab).
Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.
LOS F will result if $\mathrm{v} / \mathrm{c}>1$ irrespective of movement delay value (does not apply for approaches and intersection).
Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6).
Delay Model: HCM Delay Formula (Geometric Delay is not included).
Gap-Acceptance Capacity: Traditional M1.
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

* Critical Movement (Signal Timing)

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Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 92 | 402 | 320 |
| Average Queue (ft) | 22 | 170 | 141 |
| 95th Queue (ft) | 63 | 322 | 256 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 189 | 254 | 114 | 81 |
| Average Queue (ft) | 33 | 80 | 53 | 36 |
| 95th Queue (ft) | 113 | 209 | 94 | 68 |
| Link Distance (ft) | 475 | 251 | 2711 | 2711 |
| Upstream Blk Time (\%) |  | 0 |  |  |
| Queuing Penalty (veh) |  | 3 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 149 | 233 | 217 | 71 | 94 | 100 | 354 | 109 | 355 | 120 |
| Average Queue (ft) | 57 | 100 | 58 | 38 | 68 | 79 | 131 | 47 | 146 | 87 |
| 95th Queue (ft) | 127 | 194 | 147 | 72 | 85 | 118 | 285 | 105 | 291 | 141 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2642 |  | 1578 |  |
| Upstream Blk Time (\%) |  | 0 | 0 | 4 | 30 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 1 | 0 | 14 | 107 |  |  |  |  | 95 |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 | 16 | 7 |
| Storage Blk Time (\%) | 0 | 6 |  |  |  | 24 | 17 | 3 | 16 | 12 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T |
| Maximum Queue (ft) | 21 | 224 | 548 | 16 |
| Average Queue (ft) | 1 | 30 | 216 | 1 |
| 95th Queue (ft) | 11 | 147 | 454 | 12 |
| Link Distance (ft) | 66 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  | 0 |  |
| Queuing Penalty (veh) | 0 |  | 0 |  |
| Storage Bay Dist (ft) |  | 200 |  |  |
| Storage Blk Time (\%) |  | 0 | 10 |  |
| Queuing Penalty (veh) |  | 0 | 36 |  |

## Network Summary

Network wide Queuing Penalty: 273

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.0 | 0.0 | 0.2 | 0.2 | 0.3 | 0.1 |
| Total Del/Veh (s) | 4.9 | 2.7 | 14.4 | 13.9 | 33.6 | 28.9 | 25.2 | 16.6 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.0 | 0.2 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 35.2 | 17.8 | 9.3 | 6.1 | 36.6 | 9.4 | 11.4 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 1.8 | 0.5 | 0.5 | 3.3 | 0.6 |
| Denied Del/Veh (s) | 40.6 | 17.6 | 7.2 | 19.6 | 13.1 | 7.6 | 56.6 | 59.6 | 47.5 | 33.4 | 33.3 |
| Total Del/Veh (s) |  | 17.9 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.5 |
| Total Del/Veh (s) | 26.9 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.1 | 42.5 | 20.6 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 56.4 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 107 | 322 | 287 |
| Average Queue (ft) | 35 | 137 | 121 |
| 95th Queue (ft) | 81 | 261 | 226 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 385 | 260 | 178 | 147 |
| Average Queue (ft) | 126 | 101 | 74 | 44 |
| 95th Queue (ft) | 301 | 249 | 133 | 94 |
| Link Distance (ft) | 475 | 251 | 2711 | 2711 |
| Upstream Blk Time (\%) | 1 | 1 |  |  |
| Queuing Penalty (veh) | 2 | 8 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 277 | 256 | 67 | 108 | 100 | 594 | 104 | 209 | 118 |
| Average Queue (ft) | 94 | 158 | 112 | 22 | 74 | 59 | 308 | 40 | 67 | 61 |
| 95th Queue (ft) | 166 | 269 | 237 | 53 | 96 | 117 | 527 | 86 | 156 | 114 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2642 |  | 1578 |  |
| Upstream Blk Time (\%) |  | 1 | 0 | 1 | 49 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 4 | 1 | 4 | 157 |  |  |  |  | 95 |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 | 5 | 4 |
| Storage Blk Time (\%) | 3 | 11 |  |  |  | 6 | 52 | 1 | 5 | 9 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 25 | 6 | 225 | 596 | 103 |
| Average Queue (ft) | 0 | 0 | 31 | 364 | 17 |
| 95th Queue (ft) | 8 | 6 | 158 | 627 | 109 |
| Link Distance (ft) | 66 | 66 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 | 0 |  | 6 |  |
| Queuing Penalty (veh) | 0 | 0 |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  |  | 30 |  |
| Queuing Penalty (veh) |  |  |  | 97 |  |

Network Summary
Network wide Queuing Penalty: 387

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.3 | 0.2 | 0.3 | 0.2 | 0.2 | 0.3 |
| Total Del/Veh (s) | 5.9 | 1.9 | 16.7 | 16.0 | 56.6 | 50.3 | 46.3 | 24.1 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 36.8 | 8.6 | 7.7 | 5.0 | 51.0 | 8.8 | 7.9 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Total Del/Veh (s) | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 2.2 | 0.4 | 0.4 | 4.1 | 2.4 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 1.0 |
| Total Del/Veh (s) | 48.8 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.2 | 51.9 | 32.6 |

Total Network Performance

|  |  |
| :--- | :---: |
| Denied Del/Veh (s) | 1.4 |
| Total Del/Veh (s) | 94.7 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 127 | 440 | 445 |
| Average Queue (ft) | 29 | 223 | 202 |
| 95th Queue (ft) | 83 | 398 | 371 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 307 | 258 | 121 | 82 |
| Average Queue (ft) | 57 | 94 | 58 | 37 |
| 95th Queue (ft) | 204 | 251 | 101 | 70 |
| Link Distance (ft) | 475 | 251 | 2711 | 2711 |
| Upstream Blk Time (\%) | 0 | 1 |  |  |
| Queuing Penalty (veh) | 1 | 7 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 149 | 262 | 244 | 76 | 116 | 100 | 553 | 110 | 1124 | 120 |
| Average Queue (ft) | 72 | 141 | 100 | 52 | 74 | 85 | 229 | 47 | 648 | 106 |
| 95th Queue (ft) | 144 | 243 | 217 | 86 | 94 | 119 | 528 | 119 | 1380 | 147 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2642 |  | 1578 |  |
| Upstream BIk Time (\%) |  | 1 | 0 | 13 | 47 |  |  |  | 5 |  |
| Queuing Penalty (veh) |  | 2 | 1 | 55 | 200 |  |  |  | 0 |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 1 | 9 |  |  |  | 44 | 26 | 3 | 46 | 23 |
| Queuing Penalty (veh) | 2 | 9 |  |  |  | 59 | 29 | 11 | 141 | 63 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T |
| Maximum Queue (ft) | 13 | 225 | 672 | 749 |
| Average Queue (ft) | 1 | 108 | 540 | 281 |
| 95th Queue (ft) | 8 | 285 | 779 | 891 |
| Link Distance (ft) | 66 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  | 27 |  |
| Queuing Penalty (veh) | 0 |  | 0 |  |
| Storage Bay Dist (ft) |  | 200 |  |  |
| Storage Blk Time (\%) |  | 0 | 41 |  |
| Queuing Penalty (veh) |  | 1 | 176 |  |

Network Summary
Network wide Queuing Penalty: 759

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.3 | 0.3 | 0.0 | 0.0 | 0.2 | 0.2 | 0.2 | 0.2 |
| Total Del/Veh (s) | 9.7 | 7.2 | 22.4 | 21.2 | 47.7 | 41.6 | 42.7 | 22.7 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.3 | 0.9 | 0.1 | 0.1 | 0.1 | 0.1 | 0.4 |
| Total Del/Veh $(\mathrm{s})$ | 52.4 | 35.0 | 11.7 | 8.5 | 46.6 | 17.5 | 21.0 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 2.0 | 0.6 | 0.7 | 3.2 | 0.7 |
| Denied Del/Veh (s) | 61.2 | 24.0 | 11.0 | 32.2 | 17.8 | 10.6 | 258.2 | 261.7 | 248.6 | 38.1 | 33.1 |
| Total Del/Veh (s) |  | 17.2 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.6 |
| Total Del/Veh (s) | 87.9 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.3 | 136.3 | 56.5 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.2 |
| Total Del/Veh (s) | 214.5 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 238 | 370 | 356 |
| Average Queue (ft) | 79 | 189 | 147 |
| 95th Queue (ft) | 192 | 331 | 306 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 491 | 261 | 221 | 169 |
| Average Queue (ft) | 267 | 156 | 99 | 54 |
| 95th Queue (ft) | 512 | 318 | 184 | 122 |
| Link Distance (ft) | 475 | 251 | 2711 | 2711 |
| Upstream Blk Time (\%) | 5 | 3 |  |  |
| Queuing Penalty (veh) | 25 | 23 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 290 | 285 | 70 | 113 | 100 | 1980 | 109 | 250 | 119 |
| Average Queue (ft) | 139 | 229 | 186 | 32 | 77 | 60 | 1364 | 47 | 77 | 67 |
| 95th Queue (ft) | 174 | 315 | 316 | 69 | 99 | 123 | 2332 | 98 | 183 | 123 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2642 |  | 1578 |  |
| Upstream Blk Time (\%) |  | 9 | 2 | 5 | 64 |  | 0 |  |  |  |
| Queuing Penalty (veh) |  | 43 | 9 | 16 | 216 |  | 0 |  |  | 95 |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 | 5 | 4 |
| Storage Blk Time (\%) | 27 | 19 |  |  |  | 5 | 66 | 4 | 5 | 12 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T |
| Maximum Queue (ft) | 9 | 225 | 673 | 3206 |
| Average Queue (ft) | 0 | 63 | 639 | 1844 |
| 95th Queue (ft) | 7 | 227 | 660 | 3357 |
| Link Distance (ft) | 66 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  | 75 | 2 |
| Queuing Penalty (veh) | 0 |  | 0 | 0 |
| Storage Bay Dist (ft) |  | 200 |  |  |
| Storage Blk Time (\%) |  | 0 | 71 |  |
| Queuing Penalty (veh) |  | 0 | 236 |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 60 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 52 |
| Link Distance (ft) | 560 |
| Upstream Blk Time (\%) | 0 |
| Queuing Penalty (veh) | 0 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Network Summary

Network wide Queuing Penalty: 809

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.2 | 0.1 | 0.2 | 0.2 | 0.3 | 0.2 |
| Total Del/Veh (s) | 4.5 | 1.8 | 13.6 | 12.3 | 37.6 | 37.7 | 28.9 | 17.9 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 2.3 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 36.6 | 13.1 | 7.7 | 5.0 | 57.4 | 16.7 | 10.1 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.1 | 0.2 | 0.3 | 0.0 | 0.0 | 0.0 | 2.1 | 0.4 | 0.3 | 3.7 | 0.8 |
| Total Del/Veh (s) | 30.9 | 26.0 | 14.5 | 15.2 | 8.8 | 4.5 | 57.8 | 54.4 | 31.2 | 56.2 | 55.1 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.7 |
| Total Del/Veh (s) | 25.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.4 | 0.0 | 0.1 | 0.1 | 0.2 |
| Total Del/Veh (s) | 4.0 | 44.8 | 642.1 | 1571.5 | 30.4 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.2 |
| Total Del/Veh (s) | 73.4 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 83 | 404 | 308 |
| Average Queue (ft) | 19 | 177 | 141 |
| 95th Queue (ft) | 58 | 341 | 258 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 376 | 256 | 212 | 177 |
| Average Queue (ft) | 65 | 84 | 64 | 48 |
| 95th Queue (ft) | 233 | 221 | 159 | 132 |
| Link Distance (ft) | 475 | 251 | 2718 | 2718 |
| Upstream Blk Time (\%) | 0 | 1 |  |  |
| Queuing Penalty (veh) | 0 | 5 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 149 | 272 | 260 | 73 | 102 | 100 | 392 | 109 | 520 | 120 |
| Average Queue (ft) | 53 | 122 | 89 | 39 | 66 | 77 | 143 | 53 | 182 | 88 |
| 95th Queue (ft) | 125 | 243 | 217 | 78 | 101 | 121 | 312 | 115 | 405 | 145 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2666 |  | 1543 |  |
| Upstream BIk Time (\%) |  | 5 | 4 | 6 | 33 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 14 | 11 | 20 | 117 |  |  |  |  | 95 |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 | 18 | 10 |
| Storage Blk Time (\%) | 0 | 13 |  |  |  | 23 | 24 | 9 | 18 |  |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 71 | 70 | 225 | 661 | 1224 | 27 | 26 |
| Average Queue (ft) | 9 | 8 | 59 | 392 | 255 | 4 | 8 |
| 95th Queue (ft) | 48 | 44 | 214 | 754 | 952 | 18 | 29 |
| Link Distance (ft) | 66 | 66 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 8 | 7 |  | 20 |  |  |  |
| Queuing Penalty (veh) | 18 | 16 |  | 0 |  |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 29 |  |  |  |
| Queuing Penalty (veh) |  |  | 0 | 103 |  |  |  |

Network Summary
Network wide Queuing Penalty: 453

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.0 | 0.0 | 0.2 | 0.2 | 0.2 | 0.1 |
| Total Del/Veh (s) | 12.3 | 11.1 | 18.1 | 16.7 | 45.0 | 35.7 | 35.1 | 22.6 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.9 | 0.2 | 0.2 | 0.1 | 0.1 | 0.4 |
| Total Del/Veh $(\mathrm{s})$ | 57.0 | 41.0 | 10.1 | 7.1 | 67.6 | 21.6 | 21.1 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.2 | 0.2 | 0.0 | 0.0 | 0.0 | 1.7 | 0.5 | 0.5 | 3.8 | 0.7 |
| Denied Del/Veh (s) | 0.9 | 28.3 | 16.4 | 25.0 | 14.5 | 9.6 | 115.7 | 120.4 | 105.7 | 50.9 | 37.6 |
| Total Del/Veh (s) | 51.9 | 19.9 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.6 |
| Total Del/Veh (s) | 46.4 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.0 | 0.1 | 0.1 | 0.0 |
| Total Del/Veh (s) | 3.4 | 105.3 | 1436.1 | 714.2 | 51.6 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.0 |
| Total Del/Veh (s) | 140.0 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 216 | 390 | 332 |
| Average Queue (ft) | 54 | 158 | 145 |
| 95th Queue (ft) | 148 | 309 | 290 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 494 | 264 | 302 | 267 |
| Average Queue (ft) | 198 | 115 | 100 | 62 |
| 95th Queue (ft) | 467 | 284 | 214 | 171 |
| Link Distance (ft) | 475 | 251 | 2718 | 2718 |
| Upstream Blk Time (\%) | 7 | 1 |  |  |
| Queuing Penalty (veh) | 25 | 11 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 282 | 272 | 66 | 116 | 100 | 1127 | 109 | 308 | 120 |
| Average Queue (ft) | 92 | 183 | 140 | 22 | 73 | 65 | 539 | 46 | 82 | 63 |
| 95th Queue (ft) | 172 | 303 | 287 | 57 | 110 | 127 | 1089 | 100 | 219 | 121 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2666 |  | 1543 |  |
| Upstream Blk Time (\%) |  | 10 | 6 | 2 | 53 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 35 | 20 | 6 | 171 |  |  |  |  | 95 |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 | 5 | 4 |
| Storage Blk Time (\%) | 7 | 20 |  |  |  | 7 | 61 | 6 | 5 | 10 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 70 | 77 | 225 | 656 | 2029 | 28 | 20 |
| Average Queue (ft) | 9 | 10 | 53 | 570 | 898 | 7 | 4 |
| 95th Queue (ft) | 47 | 50 | 208 | 786 | 2370 | 26 | 21 |
| Link Distance (ft) | 66 | 66 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 6 | 8 |  | 54 |  |  |  |
| Queuing Penalty (veh) | 23 | 28 |  | 0 |  |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 62 |  |  |  |
| Queuing Penalty (veh) |  |  | 0 | 196 |  |  |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 45 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 45 |
| Link Distance (ft) | 552 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Network Summary |  |
| Network wide Queuing Penalty: 674 |  |

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.4 | 0.3 | 0.3 | 0.2 | 0.3 | 0.3 |
| Total Del/Veh (s) | 6.0 | 3.8 | 17.9 | 17.1 | 50.5 | 54.1 | 40.6 | 23.5 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.0 | 0.2 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 46.1 | 18.7 | 8.2 | 5.5 | 96.5 | 22.2 | 12.8 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.3 | 0.1 | 0.1 | 0.0 | 0.0 | 0.0 | 2.1 | 0.4 | 0.4 | 23.3 | 19.8 |
| Total Del/Veh (s) | 53.6 | 29.2 | 17.8 | 19.0 | 10.9 | 6.9 | 132.7 | 102.6 | 85.4 | 201.2 | 196.0 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 5.4 |
| Total Del/Veh (s) | 70.9 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.3 | 0.0 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh (s) | 4.0 | 77.6 | 1444.4 | 1076.6 | 51.1 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 10.1 |
| Total Del/Veh (s) | 187.5 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 124 | 476 | 420 |
| Average Queue (ft) | 28 | 242 | 187 |
| 95th Queue (ft) | 84 | 438 | 346 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  | 1 |  |
| Queuing Penalty (veh) |  | 4 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 467 | 260 | 280 | 242 |
| Average Queue (ft) | 99 | 112 | 79 | 55 |
| 95th Queue (ft) | 320 | 273 | 187 | 152 |
| Link Distance (ft) | 475 | 251 | 2718 | 2718 |
| Upstream Blk Time (\%) | 1 | 1 |  |  |
| Queuing Penalty (veh) | 3 | 9 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 287 | 280 | 72 | 115 | 100 | 600 | 109 | 1437 | 120 |
| Average Queue (ft) | 84 | 168 | 131 | 48 | 75 | 86 | 277 | 47 | 940 | 109 |
| 95th Queue (ft) | 167 | 287 | 266 | 87 | 111 | 122 | 643 | 117 | 1733 | 151 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2666 | 1543 |  |  |
| Upstream BIk Time (\%) |  | 8 | 4 | 14 | 45 |  |  |  | 17 |  |
| Queuing Penalty (veh) |  | 23 | 13 | 61 | 194 |  |  |  | 0 |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 |  | 95 |
| Storage Blk Time (\%) | 5 | 19 |  |  |  | 45 | 30 | 5 | 54 | 24 |
| Queuing Penalty (veh) | 9 | 19 |  |  |  | 60 | 33 | 24 | 165 | 66 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 77 | 69 | 225 | 663 | 3250 | 24 | 20 |
| Average Queue (ft) | 10 | 8 | 115 | 630 | 1797 | 7 | 5 |
| 95th Queue (ft) | 52 | 43 | 288 | 680 | 3693 | 32 | 21 |
| Link Distance (ft) | 66 | 66 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 8 | 7 |  | 55 | 9 |  |  |
| Queuing Penalty (veh) | 20 | 18 |  | 0 | 0 |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 55 |  |  |  |
| Queuing Penalty (veh) |  |  | 1 | 233 |  |  |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 53 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 53 |
| Link Distance (ft) | 552 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Network Summary |  |
| Network wide Queuing Penalty: 954 |  |

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 3.5 | 2.6 | 0.1 | 0.2 | 0.2 | 0.2 | 0.3 | 1.3 |
| Total Del/Veh (s) | 49.7 | 48.7 | 29.0 | 28.3 | 123.1 | 98.3 | 104.7 | 56.0 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 3.0 | 4.7 | 0.3 | 0.2 | 0.2 | 0.1 | 1.7 |
| Total Del/Veh $(\mathrm{s})$ | 87.8 | 74.2 | 13.7 | 10.5 | 64.0 | 34.1 | 39.4 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 106.4 | 101.8 | 96.5 | 3.6 | 0.7 |
| Total Del/Veh (s) | 77.7 | 32.7 | 19.3 | 31.2 | 17.7 | 10.8 | 470.7 | 483.2 | 477.8 | 63.1 | 41.2 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 27.0 |
| Total Del/Veh (s) | 152.8 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.0 | 0.1 | 0.1 | 0.0 |
| Total Del/Veh (s) | 3.2 | 146.6 | 770.9 | 805.5 | 63.0 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 32.1 |
| Total Del/Veh (s) | 322.7 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 583 | 424 | 676 |
| Average Queue (ft) | 201 | 195 | 273 |
| 95th Queue (ft) | 505 | 375 | 630 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) | 6 | 0 |  |
| Queuing Penalty (veh) | 0 | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 497 | 266 | 342 | 300 |
| Average Queue (ft) | 400 | 187 | 139 | 88 |
| 95th Queue (ft) | 615 | 340 | 264 | 213 |
| Link Distance (ft) | 475 | 251 | 2718 | 2718 |
| Upstream Blk Time (\%) | 22 | 5 |  |  |
| Queuing Penalty (veh) | 122 | 38 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | SB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | TR | L | T | R |
| Maximum Queue (ft) | 150 | 296 | 290 | 67 | 120 | 100 | 2723 | 109 | 567 | 119 |
| Average Queue (ft) | 136 | 250 | 216 | 29 | 73 | 55 | 2309 | 54 | 116 | 64 |
| 95th Queue (ft) | 188 | 329 | 332 | 63 | 104 | 119 | 3108 | 107 | 365 | 123 |
| Link Distance (ft) |  | 251 | 251 | 66 | 66 |  | 2666 |  | 1543 |  |
| Upstream Blk Time (\%) |  | 24 | 7 | 3 | 62 |  | 46 |  |  |  |
| Queuing Penalty (veh) |  | 108 | 33 | 9 | 206 |  | 0 |  |  | 95 |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 85 | 6 | 4 |
| Storage Blk Time (\%) | 41 | 24 |  |  |  | 5 | 70 | 11 | 6 | 15 |
| Queuing Penalty (veh) | 119 | 51 |  |  |  | 25 | 66 | 28 | 7 |  |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 82 | 80 | 224 | 666 | 3721 | 35 | 27 |
| Average Queue (ft) | 12 | 10 | 52 | 628 | 2374 | 8 | 8 |
| 95th Queue (ft) | 55 | 52 | 207 | 656 | 4191 | 27 | 28 |
| Link Distance (ft) | 66 | 66 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 8 | 8 |  | 76 | 23 |  |  |
| Queuing Penalty (veh) | 35 | 33 |  | 0 | 0 |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 72 |  |  |  |
| Queuing Penalty (veh) |  |  | 0 | 242 |  |  |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 170 |
| Average Queue (ft) | 12 |
| 95th Queue (ft) | 145 |
| Link Distance (ft) | 552 |
| Upstream Blk Time (\%) | 0 |
| Queuing Penalty (veh) | 0 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Network Summary

## Network wide Queuing Penalty: 1137

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.1 | 0.2 | 0.2 | 0.3 | 0.2 | 0.2 |
| Total Del/Veh (s) | 5.1 | 2.2 | 13.3 | 11.6 | 33.8 | 41.2 | 26.7 | 17.3 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.0 | 0.1 | 0.1 | 0.2 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 23.6 | 4.8 | 6.8 | 4.2 | 54.3 | 7.1 | 6.2 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Total Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.4 | 0.6 | 3.4 | 3.2 | 0.7 |
|  | 25.0 | 13.5 | 5.2 | 13.3 | 7.5 | 3.4 | 52.9 | 48.7 | 6.5 | 40.2 | 48.2 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 18.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.0 | 16.8 | 10.4 |

Total Network Performance

|  |  |
| :--- | :---: |
| Denied Del/Veh (s) | 1.1 |
| Total Del/Veh (s) | 41.2 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 89 | 396 | 291 |
| Average Queue (ft) | 21 | 170 | 136 |
| 95th Queue (ft) | 62 | 327 | 240 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 176 | 236 | 114 | 84 |
| Average Queue (ft) | 39 | 76 | 53 | 35 |
| 95th Queue (ft) | 125 | 195 | 92 | 68 |
| Link Distance (ft) | 475 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) |  | 0 |  |  |
| Queuing Penalty (veh) |  | 1 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 143 | 249 | 226 | 70 | 74 | 100 | 244 | 135 | 109 | 393 | 120 |
| Average Queue (ft) | 54 | 98 | 64 | 36 | 64 | 75 | 90 | 34 | 51 | 141 | 83 |
| 95th Queue (ft) | 121 | 202 | 164 | 69 | 75 | 118 | 204 | 93 | 110 | 303 | 137 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 0 | 0 | 6 | 30 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 0 | 0 | 22 | 109 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  | 95 |
| Storage BIk Time (\%) | 0 | 6 |  |  |  | 22 | 8 | 0 | 3 | 15 | 7 |
| Queuing Penalty (veh) | 0 | 4 |  |  |  | 28 | 14 | 0 | 10 | 35 | 12 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T |
| Maximum Queue (ft) | 11 | 224 | 510 | 18 |
| Average Queue (ft) | 1 | 29 | 220 | 1 |
| 95th Queue (ft) | 9 | 145 | 462 | 16 |
| Link Distance (ft) | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  | 0 |  |
| Queuing Penalty (veh) | 0 |  | 0 |  |
| Storage Bay Dist (ft) |  | 200 |  |  |
| Storage Blk Time (\%) |  | 0 | 10 |  |
| Queuing Penalty (veh) |  | 0 | 37 |  |

Network Summary
Network wide Queuing Penalty: 273

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.3 | 0.0 | 0.1 | 0.2 | 0.3 | 0.2 | 0.1 |
| Total Del/Veh (s) | 4.6 | 3.4 | 12.5 | 12.2 | 31.1 | 26.8 | 19.1 | 14.6 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.1 | 0.2 | 0.2 | 0.2 | 0.1 | 0.2 |
| Total Del/Veh (s) | 34.1 | 14.4 | 8.9 | 6.0 | 40.9 | 8.9 | 10.3 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.1 | 0.8 | 3.1 | 3.4 | 0.6 |
| Denied Del/Veh (s) | 30.0 | 12.3 | 4.6 | 13.9 | 9.3 | 4.9 | 46.3 | 48.2 | 9.7 | 50.4 | 45.9 |
| Total Del/Veh (s) |  | 19.8 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 20.1 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.0 | 21.9 | 10.8 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.1 |
| Total Del/Veh (s) | 43.4 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 114 | 318 | 249 |
| Average Queue (ft) | 35 | 117 | 112 |
| 95th Queue (ft) | 84 | 227 | 210 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 340 | 259 | 157 | 131 |
| Average Queue (ft) | 109 | 108 | 72 | 40 |
| 95th Queue (ft) | 256 | 255 | 127 | 84 |
| Link Distance (ft) | 475 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 0 | 1 |  |  |
| Queuing Penalty (veh) | 0 | 7 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SB |  |  |  |  |  |  |  |  |  |  |
| irections Served | L | T | TR | L | TR | L | T | $R$ | $L$ | T |
| Maximum Queue (ft) | 149 | 257 | 233 | 58 | 92 | 100 | 459 | 225 | 109 | 256 |
| Average Queue (ft) | 79 | 115 | 67 | 23 | 66 | 67 | 178 | 74 | 53 | 89 |
| 95th Queue (ft) | 148 | 215 | 173 | 52 | 79 | 122 | 338 | 187 | 107 | 197 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  | 126 |  |
| Upstream Blk Time (\%) |  | 0 | 0 | 2 | 35 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 1 | 1 | 6 | 112 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  |
| Storage Blk Time (\%) | 2 | 6 |  |  |  | 8 | 37 | 0 | 5 | 9 |
| Queuing Penalty (veh) | 5 | 7 |  |  |  | 33 | 100 | 0 | 11 | 18 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 24 | 3 | 202 | 510 | 59 |
| Average Queue (ft) | 1 | 0 | 25 | 236 | 5 |
| 95th Queue (ft) | 10 | 3 | 142 | 463 | 69 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  |  | 1 |  |
| Queuing Penalty (veh) | 0 |  |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  |  | 13 |  |
| Queuing Penalty (veh) |  |  |  | 43 |  |

Network Summary
Network wide Queuing Penalty: 350

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.4 | 0.3 | 0.3 | 0.2 | 0.2 | 0.3 |
| Total Del/Veh (s) | 5.3 | 2.4 | 17.1 | 16.5 | 53.5 | 61.9 | 41.9 | 23.5 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.1 | 0.1 | 0.1 | 0.2 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 44.0 | 10.8 | 7.6 | 5.1 | 62.4 | 9.0 | 8.4 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.4 | 0.6 | 3.5 | 3.2 | 1.1 |
| Total Del/Veh (s) | 50.6 | 19.9 | 9.2 | 18.4 | 9.7 | 5.3 | 96.7 | 73.5 | 17.5 | 123.7 | 141.2 |
| 120.7 |  |  |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.9 |
| Total Del/Veh (s) | 49.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.3 | 63.6 | 40.0 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.3 |
| Total Del/Veh (s) | 104.3 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 107 | 461 | 411 |
| Average Queue (ft) | 27 | 237 | 195 |
| 95th Queue (ft) | 74 | 418 | 348 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 2 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 343 | 258 | 136 | 93 |
| Average Queue (ft) | 72 | 98 | 62 | 36 |
| 95th Queue (ft) | 243 | 254 | 113 | 69 |
| Link Distance (ft) | 475 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 0 | 1 |  |  |
| Queuing Penalty (veh) | 1 | 6 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 150 | 273 | 255 | 74 | 95 | 100 | 430 | 224 | 110 | 1255 | 120 |
| Average Queue (ft) | 88 | 153 | 108 | 50 | 68 | 83 | 152 | 62 | 43 | 712 | 109 |
| 95th Queue (ft) | 165 | 260 | 227 | 83 | 83 | 118 | 394 | 186 | 113 | 1327 | 146 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 1 | 0 | 20 | 47 |  |  |  |  | 1 |  |
| Queuing Penalty (veh) |  | 3 | 1 | 84 | 202 |  |  |  |  | 0 |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  | 95 |
| Storage BIk Time (\%) | 5 | 11 |  |  |  | 44 | 11 | 0 | 2 | 51 | 24 |
| Queuing Penalty (veh) | 9 | 11 |  |  |  | 59 | 20 | 0 | 11 | 156 | 67 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T |
| Maximum Queue (ft) | 20 | 225 | 669 | 1087 |
| Average Queue (ft) | 1 | 118 | 584 | 420 |
| 95th Queue (ft) | 10 | 292 | 779 | 1096 |
| Link Distance (ft) | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  | 37 |  |
| Queuing Penalty (veh) | 0 |  | 0 |  |
| Storage Bay Dist (ft) |  | 200 |  |  |
| Storage Blk Time (\%) |  | 0 | 47 |  |
| Queuing Penalty (veh) |  | 0 | 200 |  |

Network Summary
Network wide Queuing Penalty: 831

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.4 | 0.4 | 0.1 | 0.1 | 0.2 | 0.2 | 0.2 | 0.2 |
| Total Del/Veh (s) | 10.0 | 8.4 | 26.2 | 25.8 | 45.9 | 45.2 | 33.1 | 24.3 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.3 | 0.1 | 0.2 | 0.2 | 0.1 | 0.2 |
| Total Del/Veh $(\mathrm{s})$ | 50.1 | 30.9 | 10.8 | 7.5 | 33.9 | 13.9 | 18.0 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.1 | 1.0 | 3.0 | 3.3 | 0.7 |
| Denied Del/Veh (s) | 46.4 | 16.8 | 7.3 | 18.5 | 12.7 | 7.6 | 90.5 | 100.1 | 50.0 | 57.8 | 44.7 |
| Total Del/Veh (s) |  | 22.2 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 36.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.3 | 79.2 | 35.6 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.2 |
| Total Del/Veh (s) | 90.8 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 285 | 438 | 294 |
| Average Queue (ft) | 82 | 218 | 151 |
| 95th Queue (ft) | 221 | 391 | 266 |
| Link Distance (ft) | 581 | 475 | 1700 |
| Upstream Blk Time (\%) | 0 | 1 |  |
| Queuing Penalty (veh) | 0 | 4 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 480 | 262 | 206 | 173 |
| Average Queue (ft) | 257 | 145 | 92 | 51 |
| 95th Queue (ft) | 505 | 308 | 161 | 115 |
| Link Distance (ft) | 475 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 3 | 2 |  |  |
| Queuing Penalty (veh) | 19 | 17 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 150 | 286 | 265 | 68 | 94 | 100 | 863 | 225 | 109 | 257 | 120 |
| Average Queue (ft) | 125 | 186 | 137 | 29 | 68 | 64 | 475 | 157 | 55 | 89 | 73 |
| 95th Queue (ft) | 178 | 310 | 283 | 61 | 81 | 126 | 906 | 292 | 110 | 209 | 128 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 4 |  | 4 | 52 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 17 | 3 | 14 | 175 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  | 95 |
| Storage BIk Time (\%) | 16 | 12 |  |  |  | 8 | 63 | 0 | 6 | 8 | 6 |
| Queuing Penalty (veh) | 47 | 26 |  |  |  | 44 | 195 | 1 | 16 | 19 | 11 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 33 | 6 | 225 | 646 | 795 |
| Average Queue (ft) | 1 | 0 | 58 | 515 | 275 |
| 95th Queue (ft) | 12 | 4 | 219 | 786 | 933 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  |  | 35 |  |
| Queuing Penalty (veh) | 0 |  |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  | 0 | 51 |  |
| Queuing Penalty (veh) |  |  | 0 | 171 |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 99 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 47 |
| Link Distance (ft) | 560 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
|  |  |
| Network Summary |  |
| Network wide Queuing Penalty: 781 |  |

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.1 | 0.2 | 0.1 | 0.2 | 0.2 | 0.2 | 0.2 |
| Total Del/Veh (s) | 4.3 | 2.6 | 13.3 | 11.9 | 37.9 | 42.4 | 27.2 | 17.9 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 2.0 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 39.1 | 11.7 | 7.5 | 5.1 | 54.5 | 15.9 | 9.6 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.1 | 0.3 | 0.2 | 0.0 | 0.0 | 0.0 | 2.1 | 0.5 | 2.0 | 3.6 | 0.8 |
| Denied Del/Veh (s) | 34.9 | 26.5 | 14.8 | 15.4 | 8.3 | 3.9 | 61.6 | 51.7 | 18.6 | 56.4 | 55.7 |
| Total Del/Veh (s) |  | 32.7 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 25.8 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.3 | 0.0 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh (s) | 3.9 | 50.6 | 910.0 | 1420.2 | 34.2 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.2 |
| Total Del/Veh (s) | 84.0 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 78 | 386 | 319 |
| Average Queue (ft) | 19 | 170 | 139 |
| 95th Queue (ft) | 57 | 326 | 255 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 330 | 252 | 227 | 206 |
| Average Queue (ft) | 59 | 79 | 66 | 48 |
| 95th Queue (ft) | 214 | 218 | 156 | 136 |
| Link Distance (ft) | 475 | 251 | 2727 | 2727 |
| Upstream Blk Time (\%) | 0 | 1 |  |  |
| Queuing Penalty (veh) | 1 | 5 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 149 | 272 | 269 | 70 | 84 | 100 | 337 | 209 | 109 | 540 | 120 |
| Average Queue (ft) | 59 | 131 | 96 | 37 | 61 | 74 | 103 | 47 | 53 | 190 | 90 |
| 95th Queue (ft) | 137 | 252 | 227 | 74 | 90 | 118 | 306 | 139 | 117 | 425 | 145 |
| Link Distance (ft) |  | 251 | 251 | 54 | 54 |  | 2642 |  |  | 1543 |  |
| Upstream Blk Time (\%) |  | 6 | 4 | 9 | 34 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 15 | 12 | 31 | 123 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  | 95 |
| Storage BIk Time (\%) | 0 | 14 |  |  |  | 26 | 9 | 0 | 7 | 19 | 11 |
| Queuing Penalty (veh) | 0 | 10 |  |  |  | 32 | 15 | 0 | 19 | 44 | 19 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 64 | 64 | 225 | 660 | 1641 | 20 | 22 |
| Average Queue (ft) | 8 | 7 | 70 | 429 | 434 | 5 | 7 |
| 95th Queue (ft) | 41 | 38 | 234 | 786 | 1427 | 19 | 25 |
| Link Distance (ft) | 54 | 54 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 7 | 6 |  | 25 |  |  |  |
| Queuing Penalty (veh) | 17 | 15 |  | 0 |  |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 34 |  |  |  |
| Queuing Penalty (veh) |  |  | 0 | 119 |  |  |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 98 |
| Average Queue (ft) | 3 |
| 95th Queue (ft) | 70 |
| Link Distance (ft) | 552 |
| Upstream Blk Time (\%) | 0 |
| Queuing Penalty (veh) | 0 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Network Summary

## Network wide Queuing Penalty: 479

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.0 | 0.0 | 0.2 | 0.2 | 0.2 | 0.1 |
| Total Del/Veh (s) | 6.7 | 4.4 | 14.8 | 13.6 | 34.2 | 33.4 | 23.4 | 17.0 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.1 | 0.1 | 0.2 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 52.4 | 33.3 | 9.0 | 6.1 | 51.0 | 19.4 | 17.7 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.1 | 0.1 | 0.2 | 0.0 | 0.0 | 0.0 | 2.0 | 0.6 | 2.0 | 3.7 | 0.6 |
| Denied Del/Veh (s) | 42.6 | 21.9 | 10.1 | 15.4 | 9.7 | 6.0 | 54.1 | 56.6 | 26.8 | 61.8 | 46.0 |
| Total Del/Veh $(\mathrm{s})$ |  | 24.2 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.7 |
| Total Del/Veh (s) | 26.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.1 | 0.0 | 0.1 | 0.1 | 0.0 |
| Total Del/Veh (s) | 3.5 | 61.8 | 1499.2 | 1079.4 | 33.1 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.0 |
| Total Del/Veh (s) | 80.4 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 130 | 350 | 263 |
| Average Queue (ft) | 40 | 136 | 119 |
| 95th Queue (ft) | 96 | 264 | 228 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 459 | 255 | 273 | 233 |
| Average Queue (ft) | 171 | 96 | 95 | 59 |
| 95th Queue (ft) | 408 | 243 | 201 | 159 |
| Link Distance (ft) | 475 | 251 | 2727 | 2727 |
| Upstream Blk Time (\%) | 3 | 1 |  |  |
| Queuing Penalty (veh) | 13 | 6 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T |
| Maximum Queue (ft) | 149 | 286 | 270 | 60 | 85 | 100 | 595 | 225 | 109 | 321 |
| Average Queue (ft) | 88 | 147 | 98 | 21 | 61 | 66 | 217 | 95 | 51 | 92 |
| 95th Queue (ft) | 169 | 276 | 240 | 51 | 93 | 123 | 474 | 224 | 104 | 231 |
| Link Distance (ft) |  | 251 | 251 | 54 | 54 |  | 2642 |  | 128 |  |
| Upstream Blk Time (\%) |  | 7 | 5 | 2 | 37 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 25 | 17 | 7 | 120 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  |
| Storage Blk Time (\%) | 4 | 15 |  |  |  | 8 | 40 | 4 | 7 | 7 |
| Queuing Penalty (veh) | 10 | 19 |  |  |  | 33 | 107 | 12 | 17 | 15 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 72 | 68 | 225 | 649 | 1099 | 29 | 27 |
| Average Queue (ft) | 11 | 8 | 46 | 419 | 299 | 7 | 6 |
| 95th Queue (ft) | 48 | 41 | 193 | 774 | 975 | 24 | 23 |
| Link Distance (ft) | 54 | 54 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 8 | 7 |  | 28 |  |  |  |
| Queuing Penalty (veh) | 29 | 26 |  | 0 |  |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 37 |  |  |  |
| Queuing Penalty (veh) |  |  | 0 | 117 |  |  |  |

Intersection: 15: Bend

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | T |  |
| Maximum Queue (ft) | 344 | 52 |
| Average Queue (ft) | 14 | 2 |
| 95th Queue (ft) | 158 | 50 |
| Link Distance (ft) | 552 | 552 |
| Upstream Blk Time (\%) | 0 |  |
| Queuing Penalty (veh) | 1 |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Network Summary

## Network wide Queuing Penalty: 582

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.3 | 0.2 | 0.3 | 0.2 | 0.2 | 0.3 |
| Total Del/Veh (s) | 5.5 | 3.2 | 16.8 | 16.1 | 48.9 | 63.0 | 40.3 | 22.5 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 1.5 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 54.7 | 19.1 | 7.7 | 5.2 | 62.3 | 19.5 | 12.2 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.5 | 0.1 | 0.1 | 0.0 | 0.0 | 0.0 | 2.1 | 0.4 | 2.1 | 9.6 | 8.8 |
| Total Del/Veh (s) | 53.8 | 28.9 | 16.9 | 18.6 | 9.7 | 4.9 | 125.5 | 97.6 | 47.0 | 167.6 | 176.0 |
| 154.5 |  |  |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 2.6 |
| Total Del/Veh (s) | 64.8 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.3 | 0.0 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh (s) | 4.1 | 82.3 | 854.7 | 733.9 | 52.5 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 16.4 |
| Total Del/Veh (s) | 204.8 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<$ LR |
| Maximum Queue (ft) | 100 | 448 | 379 |
| Average Queue (ft) | 28 | 221 | 180 |
| 95th Queue (ft) | 76 | 409 | 320 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 2 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 435 | 261 | 237 | 211 |
| Average Queue (ft) | 103 | 107 | 74 | 53 |
| 95th Queue (ft) | 325 | 268 | 170 | 144 |
| Link Distance (ft) | 475 | 251 | 2727 | 2727 |
| Upstream Blk Time (\%) | 1 | 1 |  |  |
| Queuing Penalty (veh) | 4 | 7 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| irections Served | L | T | TR | L | TR | L | T | R | L | T |
| Maximum Queue (ft) | 150 | 277 | 268 | 71 | 97 | 100 | 575 | 209 | 109 | 1396 |
| Average Queue (ft) | 86 | 160 | 126 | 44 | 65 | 82 | 209 | 66 | 53 | 851 |
| 95th Queue (ft) | 170 | 277 | 255 | 84 | 93 | 122 | 598 | 196 | 125 | 1604 |
| Link Distance (ft) |  | 251 | 251 | 54 | 54 |  | 2642 |  | 150 |  |
| Upstream BIk Time (\%) |  | 6 | 3 | 17 | 45 |  |  |  | 1543 |  |
| Queuing Penalty (veh) |  | 19 | 10 | 72 | 192 |  |  |  | 11 | 0 |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  |
| Storage Blk Time (\%) | 5 | 17 |  |  |  | 44 | 15 | 1 | 7 | 50 |
| Queuing Penalty (veh) | 9 | 17 |  |  |  | 59 | 26 | 1 | 32 | 154 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 68 | 74 | 225 | 668 | 3518 | 21 | 19 |
| Average Queue (ft) | 8 | 8 | 106 | 630 | 2270 | 4 | 4 |
| 95th Queue (ft) | 40 | 44 | 281 | 699 | 4320 | 23 | 16 |
| Link Distance (ft) | 54 | 54 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 7 | 7 |  | 57 | 27 |  |  |
| Queuing Penalty (veh) | 19 | 18 |  | 0 | 0 |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 56 |  |  |  |
| Queuing Penalty (veh) |  |  | 0 | 239 |  |  |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 101 |
| Average Queue (ft) | 3 |
| 95th Queue (ft) | 73 |
| Link Distance (ft) | 552 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Bk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Network Summary |  |
| Network wide Queuing Penalty: 939 |  |

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL2 | SBL | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 1.3 | 2.5 | 0.1 | 0.1 | 0.2 | 0.2 | 0.2 | 0.6 |
| Total Del/Veh (s) | 30.1 | 25.9 | 31.2 | 31.1 | 106.0 | 85.4 | 80.2 | 45.3 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.3 | 1.7 | 0.1 | 0.2 | 0.1 | 0.1 | 0.6 |
| Total Del/Veh $(\mathrm{s})$ | 72.5 | 53.9 | 11.9 | 8.8 | 56.9 | 23.3 | 28.8 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 1.9 | 0.7 | 1.9 | 3.6 | 0.7 |
| Denied Del/Veh (s) | 49.9 | 24.7 | 14.7 | 21.2 | 14.0 | 8.5 | 134.1 | 142.7 | 103.4 | 90.6 | 56.6 |
| Total Del/Veh (s) |  | 28.6 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.7 |
| Total Del/Veh (s) | 54.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | NBT | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.1 | 0.1 | 0.0 |
| Total Del/Veh (s) | 3.1 | 127.6 | 1029.9 | 981.5 | 55.9 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.9 |
| Total Del/Veh (s) | 187.7 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | $<\mathrm{LR}$ |
| Maximum Queue (ft) | 521 | 432 | 633 |
| Average Queue (ft) | 149 | 223 | 250 |
| 95th Queue (ft) | 411 | 404 | 574 |
| Link Distance (ft) | 581 | 475 | 1670 |
| Upstream Blk Time (\%) | 3 | 1 |  |
| Queuing Penalty (veh) | 0 | 3 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 496 | 268 | 315 | 286 |
| Average Queue (ft) | 340 | 168 | 112 | 69 |
| 95th Queue (ft) | 597 | 328 | 223 | 174 |
| Link Distance (ft) | 475 | 251 | 2727 | 2727 |
| Upstream Blk Time (\%) | 14 | 3 |  |  |
| Queuing Penalty (veh) | 78 | 22 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T |
| Maximum Queue (ft) | 150 | 292 | 275 | 65 | 90 | 100 | 1221 | 225 | 109 | 505 |
| Average Queue (ft) | 127 | 215 | 170 | 28 | 64 | 59 | 729 | 185 | 62 | 138 |
| 95th Queue (ft) | 188 | 330 | 322 | 60 | 88 | 122 | 1311 | 299 | 119 | 373 |
| Link Distance (ft) |  | 251 | 251 | 54 | 54 |  | 2642 |  | 134 |  |
| Upstream Blk Time (\%) |  | 12 | 6 | 4 | 53 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 53 | 27 | 14 | 178 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 75 |  | 200 | 85 |  |
| Storage Blk Time (\%) | 19 | 19 |  |  |  | 7 | 66 | 5 | 17 | 11 |
| Queuing Penalty (veh) | 54 | 42 |  |  |  | 35 | 205 | 22 | 43 | 25 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T | T | T |
| Maximum Queue (ft) | 69 | 65 | 225 | 660 | 3021 | 22 | 16 |
| Average Queue (ft) | 11 | 7 | 77 | 625 | 1582 | 5 | 5 |
| 95th Queue (ft) | 48 | 39 | 251 | 686 | 3325 | 19 | 28 |
| Link Distance (ft) | 54 | 54 |  | 552 | 3670 | 262 | 226 |
| Upstream Blk Time (\%) | 8 | 7 |  | 71 | 5 |  |  |
| Queuing Penalty (veh) | 34 | 31 |  | 0 | 0 |  |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |  |  |
| Storage Blk Time (\%) |  |  | 0 | 69 |  |  |  |
| Queuing Penalty (veh) |  |  | 0 | 232 |  |  |  |

Intersection: 15: Bend

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | T |  |
| Maximum Queue (ft) | 226 | 55 |
| Average Queue (ft) | 10 | 2 |
| 95th Queue (ft) | 128 | 55 |
| Link Distance (ft) | 552 | 552 |
| Upstream Blk Time (\%) | 0 | 0 |
| Queuing Penalty (veh) | 0 | 0 |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |

## Network Summary

## Network wide Queuing Penalty: 1107

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.2 | 0.1 | 0.2 | 0.4 | 0.3 | 0.2 |
| Total Del/Veh (s) | 5.0 | 2.5 | 14.2 | 12.8 | 38.8 | 31.8 | 27.5 | 18.9 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.0 | 0.1 | 0.1 | 0.2 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 34.8 | 6.0 | 8.1 | 5.2 | 66.5 | 7.7 | 7.3 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.4 | 0.6 | 3.4 | 3.4 | 0.7 |
| Denied Del/Veh (s) | 29.2 | 14.8 | 6.5 | 13.8 | 8.5 | 4.7 | 57.9 | 43.6 | 4.6 | 42.8 | 50.4 |
| Total Del/Veh (s) | 25.1 |  |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 20.2 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.1 | 22.8 | 14.1 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.1 |
| Total Del/Veh (s) | 46.4 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 91 | 401 | 332 |
| Average Queue (ft) | 20 | 181 | 155 |
| 95th Queue (ft) | 62 | 346 | 278 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 238 | 253 | 129 | 101 |
| Average Queue (ft) | 38 | 84 | 55 | 37 |
| 95th Queue (ft) | 150 | 217 | 96 | 72 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) |  | 1 |  |  |
| Queuing Penalty (veh) |  | 12 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 147 | 244 | 222 | 71 | 89 | 174 | 277 | 103 | 109 | 436 | 120 |
| Average Queue (ft) | 60 | 100 | 68 | 39 | 66 | 100 | 70 | 26 | 51 | 160 | 85 |
| 95th Queue (ft) | 128 | 201 | 162 | 73 | 79 | 177 | 185 | 74 | 114 | 333 | 143 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 0 | 0 | 7 | 35 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 1 | 0 | 24 | 127 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  | 95 |
| Storage BIk Time (\%) | 1 | 5 |  |  |  | 7 | 0 |  | 4 | 16 | 8 |
| Queuing Penalty (veh) | 1 | 4 |  |  |  | 8 | 1 |  | 12 | 38 | 14 |

Intersection: 12: Cook Road

| Movement | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T |
| Maximum Queue (ft) | 24 | 224 | 601 | 79 |
| Average Queue (ft) | 1 | 47 | 263 | 4 |
| 95th Queue (ft) | 11 | 191 | 533 | 54 |
| Link Distance (ft) | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  | 2 |  |
| Queuing Penalty (veh) | 0 |  | 0 |  |
| Storage Bay Dist (ft) |  | 200 |  |  |
| Storage Blk Time (\%) |  | 0 | 16 |  |
| Queuing Penalty (veh) |  | 0 | 56 |  |

## Network Summary

Network wide Queuing Penalty: 298

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.3 | 0.0 | 0.1 | 0.2 | 0.1 | 0.2 | 0.1 |
| Total Del/Veh (s) | 5.1 | 2.1 | 14.0 | 13.7 | 38.5 | 43.5 | 28.6 | 17.3 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh (s) | 23.5 | 10.1 | 7.8 | 5.1 | 64.2 | 11.8 | 9.4 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 3.2 | 0.8 | 3.1 | 3.3 | 0.6 |
| Denied Del/Veh (s) | 36.9 | 15.6 | 6.5 | 15.2 | 10.3 | 5.8 | 39.4 | 42.3 | 6.4 | 37.1 | 41.9 |
| Total Del/Veh (s) |  | 18.4 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total Del/Veh (s) | 19.7 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.2 | 24.4 | 12.2 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.1 |
| Total Del/Veh (s) | 44.1 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 130 | 327 | 281 |
| Average Queue (ft) | 36 | 136 | 128 |
| 95th Queue (ft) | 93 | 256 | 234 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 292 | 243 | 190 | 165 |
| Average Queue (ft) | 95 | 86 | 83 | 49 |
| 95th Queue (ft) | 231 | 200 | 148 | 109 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 0 | 0 |  |  |
| Queuing Penalty (veh) | 1 | 1 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T |
| Maximum Queue (ft) | 149 | 270 | 260 | 62 | 94 | 174 | 312 | 144 | 109 | 263 |
| Average Queue (ft) | 91 | 153 | 105 | 22 | 67 | 70 | 139 | 45 | 45 | 77 |
| 95th Queue (ft) | 163 | 264 | 225 | 52 | 81 | 153 | 251 | 87 | 95 | 186 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  | 125 |  |
| Upstream Blk Time (\%) |  | 1 | 0 | 2 | 40 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 3 | 1 | 6 | 127 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  |
| Storage Blk Time (\%) | 4 | 9 |  |  |  | 0 | 9 | 0 | 3 | 6 |
| Queuing Penalty (veh) | 10 | 12 |  |  |  | 0 | 23 | 0 | 6 | 13 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 19 | 6 | 204 | 512 | 45 |
| Average Queue (ft) | 1 | 0 | 22 | 278 | 4 |
| 95th Queue (ft) | 12 | 4 | 130 | 469 | 70 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  |  | 1 |  |
| Queuing Penalty (veh) | 0 |  |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  |  | 16 |  |
| Queuing Penalty (veh) |  |  |  | 50 |  |

Network Summary
Network wide Queuing Penalty: 261

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.3 | 0.3 | 0.2 | 0.2 | 0.2 | 0.3 |
| Total Del/Veh (s) | 5.0 | 2.1 | 17.0 | 16.5 | 54.2 | 52.9 | 39.2 | 23.3 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.1 | 0.0 | 0.1 | 0.1 | 0.1 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 47.4 | 10.0 | 8.5 | 5.4 | 61.0 | 9.7 | 8.9 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.5 | 0.5 | 3.5 | 3.1 | 0.9 |
| Total Del/Veh (s) | 52.9 | 19.3 | 8.3 | 21.1 | 10.2 | 5.5 | 73.7 | 38.1 | 4.9 | 68.7 | 83.9 |
| 62.4 |  |  |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.8 |
| Total Del/Veh $(\mathrm{s})$ | 33.3 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.2 | 73.1 | 45.3 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.3 |
| Total Del/Veh (s) | 109.5 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 115 | 469 | 393 |
| Average Queue (ft) | 24 | 227 | 194 |
| 95th Queue (ft) | 72 | 426 | 338 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 2 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 330 | 260 | 147 | 127 |
| Average Queue (ft) | 66 | 114 | 63 | 41 |
| 95th Queue (ft) | 216 | 237 | 116 | 84 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 0 | 1 |  |  |
| Queuing Penalty (veh) | 0 | 9 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 154 | 261 | 249 | 74 | 91 | 174 | 300 | 103 | 109 | 815 | 120 |
| Average Queue (ft) | 87 | 134 | 95 | 51 | 67 | 107 | 80 | 29 | 46 | 431 | 107 |
| 95th Queue (ft) | 160 | 241 | 211 | 83 | 79 | 185 | 218 | 87 | 117 | 828 | 146 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 1 | 0 | 21 | 50 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 4 | 1 | 91 | 213 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  | 95 |
| Storage BIk Time (\%) | 6 | 9 |  |  |  | 11 | 1 |  | 3 | 41 | 19 |
| Queuing Penalty (veh) | 12 | 9 |  |  |  | 15 | 1 |  | 14 | 125 | 54 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 20 | 3 | 225 | 672 | 1821 |
| Average Queue (ft) | 1 | 0 | 103 | 610 | 845 |
| 95th Queue (ft) | 14 | 3 | 276 | 768 | 2057 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 |  |  | 47 |  |
| Queuing Penalty (veh) | 0 |  |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  | 0 | 52 |  |
| Queuing Penalty (veh) |  |  | 0 | 220 |  |

Network Summary
Network wide Queuing Penalty: 770

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 6.4 | 12.0 | 0.3 | 0.2 | 0.2 | 0.2 | 0.2 | 2.4 |
| Total Del/Veh (s) | 25.7 | 28.0 | 30.6 | 30.2 | 71.7 | 85.2 | 63.7 | 36.5 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 2.0 | 0.2 | 0.1 | 0.2 | 0.2 | 0.7 |
| Total Del/Veh (s) | 56.3 | 47.1 | 10.1 | 7.2 | 167.8 | 77.0 | 36.9 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.1 | 1.0 | 3.1 | 3.4 | 0.7 |
| Denied Del/Veh (s) | 88.5 | 23.8 | 11.7 | 19.4 | 11.2 | 6.5 | 58.2 | 56.5 | 11.1 | 45.8 | 39.1 |
| Total Del/Veh (s) |  | 19.8 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.8 |
| Total Del/Veh $(\mathrm{s})$ | 30.8 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 1.3 | 35.2 | 16.3 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 2.4 |
| Total Del/Veh (s) | 83.8 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 332 | 450 | 372 |
| Average Queue (ft) | 126 | 223 | 181 |
| 95th Queue (ft) | 359 | 421 | 375 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) | 4 | 2 |  |
| Queuing Penalty (veh) | 0 | 10 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 479 | 248 | 425 | 397 |
| Average Queue (ft) | 289 | 128 | 212 | 172 |
| 95th Queue (ft) | 575 | 239 | 491 | 449 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 12 | 1 |  |  |
| Queuing Penalty (veh) | 65 | 12 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 150 | 295 | 282 | 68 | 94 | 174 | 569 | 349 | 109 | 255 | 120 |
| Average Queue (ft) | 139 | 239 | 206 | 27 | 68 | 98 | 252 | 90 | 54 | 86 | 70 |
| 95th Queue (ft) | 178 | 331 | 334 | 58 | 82 | 202 | 446 | 262 | 104 | 194 | 126 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 23 | 6 | 4 | 44 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 102 | 26 | 14 | 147 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  | 95 |
| Storage BIk Time (\%) | 49 | 15 |  |  |  | 1 | 30 | 0 | 4 | 7 | 5 |
| Queuing Penalty (veh) | 143 | 31 |  |  |  | 6 | 94 | 0 | 10 | 15 | 9 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 31 | 10 | 225 | 560 | 149 |
| Average Queue (ft) | 2 | 0 | 44 | 331 | 45 |
| 95th Queue (ft) | 14 | 7 | 189 | 582 | 315 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 0 | 0 |  | 6 |  |
| Queuing Penalty (veh) | 0 | 0 |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  | 0 | 23 |  |
| Queuing Penalty (veh) |  |  | 0 | 77 |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 114 |
| Average Queue (ft) | 7 |
| 95th Queue (ft) | 110 |
| Link Distance (ft) | 560 |
| Upstream Blk Time (\%) | 0 |
| Queuing Penalty (veh) | 0 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Network Summary

## Network wide Queuing Penalty: 763

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.2 | 0.2 | 0.3 | 0.2 | 0.3 | 0.3 | 0.2 |
| Total Del/Veh (s) | 5.6 | 2.2 | 14.1 | 12.7 | 40.9 | 36.4 | 28.6 | 19.4 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.1 | 0.1 | 0.1 | 0.2 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 36.3 | 12.7 | 8.2 | 5.2 | 72.0 | 13.9 | 9.8 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.1 | 0.1 | 0.2 | 0.0 | 0.0 | 0.0 | 3.4 | 0.6 | 3.3 | 3.3 | 0.7 |
| Total Del/Veh (s) | 39.5 | 25.5 | 11.9 | 16.2 | 8.8 | 4.0 | 54.9 | 42.7 | 16.1 | 61.8 | 57.1 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.8 |
| Total DelVeh (s) | 25.1 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.4 | 0.0 | 0.2 |
| Total Del/Veh (s) | 3.9 | 50.9 | 32.2 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.3 |
| Total Del/Veh (s) | 79.0 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 100 | 428 | 315 |
| Average Queue (ft) | 22 | 176 | 161 |
| 95th Queue (ft) | 66 | 347 | 278 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 412 | 256 | 189 | 162 |
| Average Queue (ft) | 54 | 83 | 64 | 45 |
| 95th Queue (ft) | 220 | 207 | 133 | 107 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 0 | 1 |  |  |
| Queuing Penalty (veh) | 1 | 10 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 149 | 276 | 262 | 71 | 96 | 174 | 255 | 152 | 109 | 558 | 120 |
| Average Queue (ft) | 68 | 121 | 86 | 39 | 62 | 96 | 68 | 32 | 54 | 185 | 85 |
| 95th Queue (ft) | 147 | 244 | 209 | 74 | 94 | 172 | 179 | 93 | 117 | 430 | 143 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 5 | 3 | 9 | 36 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 14 | 7 | 33 | 130 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  | 95 |
| Storage BIk Time (\%) | 2 | 14 |  |  |  | 6 | 1 |  | 9 | 18 | 9 |
| Queuing Penalty (veh) | 3 | 10 |  |  |  | 7 | 1 |  | 25 | 41 | 15 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 67 | 64 | 224 | 664 | 1407 |
| Average Queue (ft) | 8 | 5 | 66 | 418 | 341 |
| 95th Queue (ft) | 39 | 31 | 227 | 763 | 1212 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 8 | 7 |  | 22 |  |
| Queuing Penalty (veh) | 19 | 16 |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  | 0 | 34 |  |
| Queuing Penalty (veh) |  |  | 0 | 121 |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 58 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 58 |
| Link Distance (ft) | 560 |
| Upstream Blk Time (\%) | 0 |
| Queuing Penalty (veh) | 0 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Network Summary

## Network wide Queuing Penalty: 454

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.3 | 0.1 | 0.1 | 0.2 | 0.1 | 0.2 | 0.2 |
| Total Del/Veh (s) | 8.8 | 4.2 | 15.9 | 15.6 | 41.6 | 38.8 | 32.5 | 20.0 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.0 | 0.1 | 0.1 | 0.1 | 0.2 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 44.3 | 31.5 | 8.6 | 5.7 | 77.3 | 30.7 | 19.8 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Total Del/Veh (s) | 0.1 | 0.1 | 0.2 | 0.0 | 0.0 | 0.1 | 3.2 | 0.8 | 3.1 | 3.3 | 0.6 |
|  | 60.5 | 26.5 | 15.9 | 17.9 | 10.2 | 7.0 | 45.4 | 46.2 | 24.4 | 67.0 | 46.3 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.9 |
| Total Del/Veh (s) | 27.9 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 3.6 | 60.1 | 30.1 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 1.1 |
| Total Del/Veh (s) | 82.0 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 185 | 360 | 287 |
| Average Queue (ft) | 45 | 146 | 136 |
| 95th Queue (ft) | 128 | 289 | 247 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 480 | 254 | 343 | 318 |
| Average Queue (ft) | 167 | 109 | 119 | 84 |
| 95th Queue (ft) | 420 | 231 | 263 | 228 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 4 | 0 |  |  |
| Queuing Penalty (veh) | 15 | 1 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T |
| Maximum Queue (ft) | 150 | 289 | 282 | 67 | 101 | 174 | 493 | 344 | 109 | 397 |
| Average Queue (ft) | 96 | 178 | 140 | 22 | 67 | 77 | 155 | 70 | 49 | 102 |
| 95th Queue (ft) | 176 | 304 | 288 | 55 | 94 | 165 | 329 | 204 | 102 | 299 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  | 118 |  |
| Upstream Blk Time (\%) |  | 11 | 7 | 3 | 40 |  |  |  |  |  |
| Queuing Penalty (veh) |  | 40 | 24 | 9 | 127 |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  |
| Storage Blk Time (\%) | 13 | 18 |  |  |  | 0 | 10 | 1 | 9 | 7 |
| Queuing Penalty (veh) | 32 | 24 |  |  |  | 1 | 28 | 5 | 21 | 14 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 74 | 66 | 224 | 668 | 1198 |
| Average Queue (ft) | 9 | 6 | 37 | 420 | 282 |
| 95th Queue (ft) | 45 | 34 | 171 | 742 | 1009 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 8 | 7 |  | 24 |  |
| Queuing Penalty (veh) | 30 | 27 |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  | 0 | 36 |  |
| Queuing Penalty (veh) |  |  | 0 | 115 |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 226 |
| Average Queue (ft) | 12 |
| 95th Queue (ft) | 144 |
| Link Distance (ft) | 560 |
| Upstream Blk Time (\%) | 0 |
| Queuing Penalty (veh) | 0 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Network Summary

Network wide Queuing Penalty: 519

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.4 | 0.3 | 0.2 | 0.2 | 0.2 | 0.3 |
| Total Del/Veh (s) | 6.5 | 2.8 | 16.8 | 16.8 | 55.6 | 62.6 | 41.0 | 23.8 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBT | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.1 | 0.2 | 0.1 | 0.1 | 0.2 | 0.1 | 0.1 |
| Total Del/Veh $(\mathrm{s})$ | 56.3 | 18.3 | 8.8 | 5.7 | 67.1 | 16.1 | 12.0 |

3: Old Highway 99 Road \& Cook Road Performance by movement

|  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | 0.0 | 0.3 | 0.2 | 0.0 | 0.0 | 0.0 | 3.5 | 0.6 | 3.4 | 6.3 | 5.0 |
| Denied Del/Veh (s) | 56.9 | 28.4 | 15.6 | 22.6 | 10.1 | 6.2 | 76.9 | 43.7 | 15.9 | 125.8 | 120.9 |
| Total Del/Veh (s) |  | 97.7 |  |  |  |  |  |  |  |  |  |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 1.9 |
| Total Del/Veh $(\mathrm{s})$ | 46.6 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.4 | 0.0 | 0.2 |
| Total Del/Veh (s) | 3.8 | 84.0 | 51.9 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 5.4 |
| Total Del/Veh (s) | 165.9 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 125 | 459 | 417 |
| Average Queue (ft) | 27 | 221 | 194 |
| 95th Queue (ft) | 80 | 422 | 348 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 443 | 260 | 194 | 181 |
| Average Queue (ft) | 96 | 122 | 72 | 49 |
| 95th Queue (ft) | 315 | 245 | 148 | 119 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 2 | 1 |  |  |
| Queuing Penalty (veh) | 6 | 10 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 156 | 265 | 264 | 73 | 96 | 173 | 324 | 161 | 109 | 1207 | 120 |
| Average Queue (ft) | 85 | 142 | 105 | 50 | 62 | 110 | 95 | 41 | 52 | 602 | 104 |
| 95th Queue (ft) | 162 | 257 | 234 | 84 | 93 | 190 | 269 | 142 | 125 | 1336 | 153 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 7 | 4 | 21 | 45 |  |  |  |  | 7 |  |
| Queuing Penalty (veh) |  | 20 | 12 | 91 | 192 |  |  |  |  | 0 |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  | 95 |
| Storage BIk Time (\%) | 7 | 16 |  |  |  | 14 | 1 | 0 | 9 | 42 | 19 |
| Queuing Penalty (veh) | 14 | 16 |  |  |  | 19 | 2 | 0 | 42 | 127 | 53 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 71 | 65 | 225 | 673 | 3391 |
| Average Queue (ft) | 7 | 5 | 115 | 615 | 1724 |
| 95th Queue (ft) | 40 | 31 | 286 | 768 | 3999 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 8 | 6 |  | 53 | 14 |
| Queuing Penalty (veh) | 19 | 15 |  | 0 | 0 |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  | 0 | 55 |  |
| Queuing Penalty (veh) |  |  | 1 | 236 |  |

Intersection: 15: Bend

| Movement | EB |
| :--- | ---: |
| Directions Served | T |
| Maximum Queue (ft) | 165 |
| Average Queue (ft) | 6 |
| 95th Queue (ft) | 98 |
| Link Distance (ft) | 560 |
| Upstream Blk Time (\%) | 0 |
| Queuing Penalty (veh) | 0 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Network Summary

## Network wide Queuing Penalty: 878

1: I-5 SB Ramp \& Cook Road Performance by movement

| Movement | EBT | EBR | WBL | WBT | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh (s) | 13.9 | 8.2 | 0.5 | 0.7 | 0.3 | 0.1 | 0.2 | 5.0 |
| Total Del/Veh (s) | 58.0 | 61.0 | 46.4 | 42.8 | 166.8 | 167.3 | 144.7 | 73.6 |

2: I-5 NB Ramp \& Cook Road Performance by movement

| Movement | EBL | EBT | WBT | WBR | NBL | NBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Denied Del/Veh $(\mathrm{s})$ | 0.9 | 5.2 | 0.4 | 0.3 | 0.3 | 0.2 | 1.8 |
| Total Del/Veh $(\mathrm{s})$ | 71.9 | 67.3 | 13.8 | 10.3 | 367.2 | 135.1 | 59.1 |

## 3: Old Highway 99 Road \& Cook Road Performance by movement

|  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 3.0 | 1.0 | 3.0 | 3.2 | 0.7 |
| Total Del/Veh (s) | 93.8 | 33.1 | 19.2 | 23.0 | 11.9 | 5.8 | 79.0 | 67.7 | 30.7 | 72.6 | 54.6 |

3: Old Highway 99 Road \& Cook Road Performance by movement

| Movement | All |
| :--- | ---: |
| Denied Del/Veh (s) | 0.9 |
| Total Del/Veh (s) | 40.5 |

12: Cook Road Performance by movement

| Movement | EBT | WBT | All |
| :--- | ---: | ---: | ---: |
| Denied Del/Veh (s) | 0.0 | 0.0 | 0.0 |
| Total Del/Veh (s) | 3.3 | 79.1 | 35.9 |

Total Network Performance

|  |  |
| :--- | ---: |
| Denied Del/Veh (s) | 4.2 |
| Total Del/Veh (s) | 155.2 |

Intersection: 1: I-5 SB Ramp \& Cook Road

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | TR | LT | LTR |
| Maximum Queue (ft) | 540 | 490 | 726 |
| Average Queue (ft) | 224 | 267 | 336 |
| 95th Queue (ft) | 559 | 521 | 769 |
| Link Distance (ft) | 574 | 474 | 1699 |
| Upstream Blk Time (\%) | 12 | 6 |  |
| Queuing Penalty (veh) | 0 | 32 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: I-5 NB Ramp \& Cook Road

| Movement | EB | WB | NB | NB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | LT | TR | LTR | R |
| Maximum Queue (ft) | 497 | 267 | 598 | 578 |
| Average Queue (ft) | 360 | 151 | 305 | 266 |
| 95th Queue (ft) | 628 | 282 | 702 | 667 |
| Link Distance (ft) | 474 | 250 | 1722 | 1722 |
| Upstream Blk Time (\%) | 23 | 5 |  |  |
| Queuing Penalty (veh) | 128 | 40 |  |  |
| Storage Bay Dist (ft) |  |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

## Intersection: 3: Old Highway 99 Road \& Cook Road

| Movement | EB | EB | EB | WB | WB | NB | NB | NB | SB | SB | SB |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Directions Served | L | T | TR | L | TR | L | T | R | L | T | R |
| Maximum Queue (ft) | 150 | 301 | 297 | 71 | 89 | 174 | 679 | 374 | 109 | 444 | 120 |
| Average Queue (ft) | 132 | 249 | 222 | 27 | 65 | 99 | 311 | 119 | 57 | 134 | 76 |
| 95th Queue (ft) | 197 | 332 | 336 | 62 | 89 | 203 | 692 | 328 | 112 | 375 | 134 |
| Link Distance (ft) |  | 250 | 250 | 53 | 53 |  | 1639 |  |  | 1578 |  |
| Upstream Blk Time (\%) |  | 32 | 13 | 5 | 44 |  |  |  |  |  |  |
| Queuing Penalty (veh) |  | 144 | 58 | 18 | 149 |  |  |  |  |  |  |
| Storage Bay Dist (ft) | 125 |  |  |  |  | 150 |  | 350 | 85 |  | 95 |
| Storage BIk Time (\%) | 47 | 23 |  |  |  | 3 | 34 | 1 | 10 | 10 | 9 |
| Queuing Penalty (veh) | 137 | 49 |  |  |  | 17 | 104 | 4 | 25 | 22 | 15 |

Intersection: 12: Cook Road

| Movement | EB | EB | WB | WB | B15 |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Directions Served | T | T | T | T | T |
| Maximum Queue (ft) | 81 | 67 | 224 | 661 | 1922 |
| Average Queue (ft) | 10 | 6 | 50 | 469 | 693 |
| 95th Queue (ft) | 49 | 33 | 202 | 786 | 2039 |
| Link Distance (ft) | 53 | 53 |  | 560 | 3647 |
| Upstream Blk Time (\%) | 8 | 8 |  | 37 |  |
| Queuing Penalty (veh) | 36 | 33 |  | 0 |  |
| Storage Bay Dist (ft) |  |  | 200 |  |  |
| Storage Blk Time (\%) |  |  | 0 | 46 |  |
| Queuing Penalty (veh) |  |  | 0 | 153 |  |

Intersection: 15: Bend

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | T |  |
| Maximum Queue (ft) | 230 | 56 |
| Average Queue (ft) | 10 | 2 |
| 95th Queue (ft) | 133 | 56 |
| Link Distance (ft) | 560 | 560 |
| Upstream Blk Time (\%) | 0 |  |
| Queuing Penalty (veh) | 0 |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Network Summary

## Network wide Queuing Penalty: 1164

## Appendix E: Safety Analysis

| General Information | Cook Road ICE |
| :--- | :--- |
| Project Name | ICE - No Action |
| Project Description | Cook Road |
| Reference Number | Transpo Group |
| Analyst | WSDOT |
| Agency/Company | 0 |
| Contact Email | - |
| Contact Phone | $01 / 00 / 00$ |
| Date Completed |  |
| PROJECT SUMMARY |  |

## Summary of Anticipated Safety Performance of the Project (average crashes/yr)



|  | Total Crashes/yr <br> (KABCO) | Fatal and Injury Crashes/yr <br> (KABC) | Property <br> Damage Only Crashes/yr (PDO) |
| :---: | :---: | :---: | :---: |
| Project Element | Predicted average crash frequency <br> $\mathrm{N}_{\text {predicted (KABCO) }}$ | Predicted average crash frequency <br> $\mathrm{N}_{\text {predicted (KABC) }}$ | Predicted average crash frequency <br> $\mathrm{N}_{\text {predicted (0) }}$ |
| INDIVIDUAL INTERSECTIONS |  |  |  |
| Intersection 1 | 1.5 | 0.6 | 0.9 |
| Intersection 2 | 2.4 | 0.9 | 1.5 |
| Intersection 3 | 2.1 | 0.7 | 1.4 |
| COMBINED (sum of column) | 6.0 | 2.2 | 3.8 |


| PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Urban and Suburban Arterial Project |  |
| :--- | :--- |
| Crash severity level | $\mathbf{N}_{\text {predicted(PRoJECT) }}$ <br>  <br> Predicted average crash <br> frequency - Average safety <br> performance of projects <br> consisting of similar elements <br> (anticipated average crashes/yr) |
| Fatal and injury (KABC) | 2.2 |
| Property damage only (PDO) | 3.8 |
| Total (KABCO) | 6.0 |

HSM1 Extended Spreadsheet for Part C Chapter 12 v. 9

Discussion of Results
Given the potential effects of project characteristics on safety performance, results indicate that:

1. It is anticipated that a typical project such as this will, on average, experience 6 crashes per year ( 2.2 fatal and injury crashes per year; and 3.8 property damage only crashes per year).

| General Information | Cook Road ICE |
| :--- | :--- |
| Project Name | ICE-Alt 2 (Traffic Signal) |
| Project Description | Cook Road |
| Reference Number | Transpo Group |
| Analyst |  |
| Agency/Company | WSDOT |
| Contact Email | 0 |
| Contact Phone | $-01 / 00 / 00$ |
| Date Completed |  |
| PROJECT SUMMARY |  |

## Summary of Anticipated Safety Performance of the Project (average crashes/yr)



|  | Total Crashes/yr |  |  |
| :--- | :---: | :--- | :--- |
| (KABCO) | Fatal and Injury <br> Crashes/yr <br> (KABC) | Property <br> Damage Only <br> Crashes/yr <br> (PDO) |  |
| Predicted <br> average crash <br> frequency | Predicted <br> average crash <br> frequency <br> $N_{\text {predicted (KABCO) }}$ | Predicted <br> average crash <br> frequency |  |
| INDIVIDUAL INTERSECTIONS (KABC) |  | $\mathrm{N}_{\text {predicted (0) }}$ |  |
| Intersection 1 | 1.3 | 0.4 | 0.9 |
| Intersection 2 | 2.3 | 0.7 | 1.5 |
| Intersection 3 | 2.1 | 0.7 | 1.4 |
| COMBINED (sum of column) | 5.7 | 1.8 | 3.9 |


| PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Urban and Suburban Arterial Project |  |
| :--- | :--- |
| Crash severity level | $\mathbf{N}_{\text {predicted(PRoJECT) }}$ <br>  <br> Patal and injury (KABC)Predicted average crash <br> frequency - Average safety <br> performance of projects <br> consisting of similar elements <br> (anticipated average crashes/yr) |
| Property damage only (PDO) | 1.8 |
| Total (KABCO) | 3.9 |

HSM1 Extended Spreadsheet for Part C Chapter 12 v. 9

Discussion of Results
Given the potential effects of project characteristics on safety performance, results indicate that:

1. It is anticipated that a typical project such as this will, on average, experience 5.7 crashes per year ( 1.8 fatal and injury crashes per year; and 3.9 property damage only crashes per year).

| General Information | Cook Road ICE |
| :--- | :--- |
| Project Name | ICE-Alt 3/4 (Traffic Signal) |
| Project Description | Cook Road |
| Reference Number | Transpo Group |
| Analyst |  |
| Agency/Company | WSDOT |
| Contact Email | 0 |
| Contact Phone | $-01 / 00 / 00$ |
| Date Completed |  |
| PROJECT SUMMARY |  |

Summary of Anticipated Safety Performance of the Project (average crashes/yr)


| Project Element | Total Crashes/yr | Fatal and Injury <br> Crashes/yr <br> (KABCO) | Property <br> (KABC) |
| :--- | :---: | :--- | :--- |
|  | Predicted <br> average crash <br> frequency | Predicted <br> average crash <br> frequency |  |
|  | $\mathrm{N}_{\text {predicted (KABC) }}$ | $\mathrm{N}_{\text {predicted (0) }}$ |  |
| INDIVIDUAL INTERSECTIONS |  |  |  |
| Intersection 1 | 1.3 | 0.4 | 0.9 |
| Intersection 2 | 2.3 | 0.7 | 1.5 |
| Intersection 3 | 2.0 | 0.7 | 1.4 |
| COMBINED (sum of column) | 5.6 | 1.8 | 3.8 |


| PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Urban and Suburban Arterial Project |  |
| :--- | ---: |
| Crash severity level | $\mathbf{N}_{\text {predicted(PRoJECT) }}$ <br>  <br> Predicted average crash <br> frequency - Average safety <br> performance of projects <br> consisting of similar elements <br> (anticipated average crashes/yr) |
| Froperty damage only (PDO) | 1.8 |
| Total (KABCO) | 3.8 |

HSM1 Extended Spreadsheet for Part C Chapter 12 v. 9

Discussion of Results
Given the potential effects of project characteristics on safety performance, results indicate that:

1. It is anticipated that a typical project such as this will, on average, experience 5.6 crashes per year ( 1.8 fatal and injury crashes per year; and 3.8 property damage only crashes per year).

[^0]:    ${ }^{1}$ Estimated based in the weekday PM peak hour traffic volumes along Cook Road between I-5 NB Ramps and Old Highway 99 S.

[^1]:    ${ }^{2}$ WSDOT Sidra Policy Settings (October 2020) and WSDOT Synchro \& Simtraffic Protocol - August 2018

[^2]:    ${ }^{3}$ Mainlines included collisions reported within 0.75 miles ( $\sim 4,000$ feet) of the ramp milepost.

[^3]:    ${ }^{4}$ Sergeant Jon McKee, \#165, Washington State Patrol, Field Operations Bureau - Burlington
    ${ }^{5}$ http://safetyperformance.org/tools/
    ${ }^{6}$ CMF ID: 4257. Conversion of signalized intersection into single urban or suburban roundabout.

